DESIGN CODE REFERENCES

THE STRUCTURE IS DESIGN IN ACCORDANCE WITH THE FOLLOWING BUILDING AND DESIGN CODES.
- 2021 INTERNATIONAL BUILDING CODE (IBC 2021, FLORIDA EDITION)
- MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDING AND OTHER STRUCTURES (ASCE 7-16 WITH SUPPLEMENT 1)
- BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-19)
- SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301-16)
- PCI DESIGN HANDBOOK - PRECAST AND PRESTRESSED CONCRETE (8TH EDITION)
- BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (TMS 402-2016)
- SPECIFICATION FOR MASONRY STRUCTURES (TMS 602-2016)
- SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS (AISC 360-16,15TH EDITION STRUCTURAL STEEL MANUAL)
- NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL NONSTRUCTURAL FRAMING (AISI S220-20)
- STANDARD SPECIFICATION LOAD TABLES AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS (SJI100-20)
- DESIGN MANUAL FOR FLOOR DECK AND ROOF DECKS (SDI)

GENERAL CONSTRUCTION NOTES

1. WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE "INTERNATIONAL BUILDING CODE" AND ALL FEDERAL, STATE AND CITY LAWS, BYLAWS, ORDINANCES AND REGULATIONS IN ANY MANNER AFFECTING THE CONDUCT OF THIS WORK AS WELL AS ALL ORDERS OR DEGREES WHICH HAVE BEEN PROMULGATED OR ENACTED BY ANY LEGAL BODIES OR TRIBUNALES HAVING AUTHORITY OR JURISDICTION OVER THE WORK, MATERIALS, EMPLOYEES OR CONTRACT
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING SAFETY OF ALL PERSONNEL ON THE JOBSITE. GUIDELINES FOR CONSTRUCTION SAFETY SHALL BE IN ACCORDANCE WITH, BUT NOT LIMITED TO, THE CONSTRUCTION INDUSTRY OSHA SAFETY AND HEALTH STANDARDS, AND ANY LOCAL ORDINANCES OR CODES WHICH MAY BE APPLICABLE
3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL AND MECHANICAL DRAWINGS, AS WELL AS ALL SPECIFICATIONS. IF THERE IS A DISCREPANCY BETWEEN DRAWINGS, IT IS THE CONTRACTORS RESPONSIBILITY TO NOTIFY THE ARCHITECT PRIOR TO PERFORMING WORK.
4. IN CASE OF CONFLICT BETWEEN THE GENERAL NOTES, SPECIFICATIONS AND DETAILS, THE MOST RIGID REQUIREMENTS SHALL GOVERN.
5. DO NOT SCALE DRAWINGS TO OBTAIN DIMENSIONAL INFORMATION.
6. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR WATER/DAMPPROOFING AND FIREPROOFING ASSEMBLIES.
7. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF MASONRY AND DRYWALL NON-LOAD BEARING PARTITIONS. PROVIDE SLIP CONNECTIONS THAT ALLOW VERTICAL MOVEMENT AT THE TOP OF ALL SUCH PARTITIONS. CONNECTIONS SHALL BE DESIGNED TO LATERALLY SUPPORT THE TOP OF THE WALLS FOR THE CODE-REQUIRED LOAD.
8. ALL COSTS OF INVESTIGATION AND/OR REDESIGN DUE TO IMPROPER INSTALLATION OF STRUCTURAL ELEMENTS BY THE CONTRACTOR OR OTHER ITEMS NOT IN CONFORMANCE WITH THE CONTRACT DOCUMENTS SHALL BE AT THE CONTRACTORS EXPENSE.
9. THE CONTRACTOR SHALL COORDINATE PRINCIPAL OPENINGS (SLEEVES, CURBS, INSERTS,SHAFTS, ETC.) IN THE STRUCTURE AS INDICATED ON THE CONTRACT DOCUMENTS, WHICH INCLUDE BUT ARE NOT LIMITED TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS. THE LOCATION OF SLEEVES OR OPENINGS IN THE STRUCTURAL MEMBERS NOT INDICATED ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER BEFORE INSTALLATION
10. THE CONTRACTOR SHALL PROVIDE BRACING AS REQUIRED TO MAINTAIN PLUMBNESS AND STABILITY DURING CONSTRUCTION OF BOTH NEW AND EXISTING STRUCTURE.
11. METHODS, PROCEDURES AND THE SEQUENCES (OTHER THAN THAT NOTED ON THE DRAWINGS) OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION AND COORDINATION OF WORK WITH MECHANICAL AND ELECTRICAL WORK.
12. AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOBSITE INCLUDING SAFETY OF PERSONS AND PROPERTY. THE ARCHITECT'S OR ENGINEER'S PRESENCE OR REVIEW OF WORK DOES NOT INCLUDE THE ADEQUACY OF THE CONTRACTOR'S MEANS OR METHODS OF CONSTRUCTION.
13. SHORING, BRACING AND PROTECTION OF EXISTING ADJACENT STRUCTURES (INCLUDING STREETS, BUILDINGS, AND STRUCTURES) DURING CONSTRUCTION IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
14. WORK NOT INDICATED ON A PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING PLACES SHALL BE PROVIDED BY THE CONTRACTOR AT NO ADDITIONAL COST.
15. MINOR DETAILS OR INCIDENTAL ITEMS NOT SHOWN OR SPECIFICALLY INDICATED ON DRAWINGS, BUT NECESSARY FOR A PROPER AND COMPLETE INSTALLATION SHALL BE PROVIDED AS REQUIRED SUCH AS MISCELLANEOUS WOOD OR COLD FORMED STEEL BLOCKING, FRAMING MEMBERS, ANCHORS, FASTENERS, ETC

PERFORMANCE ASSEMBLIES/ STRUCTURAL COMPONENT DESIGN SUBMITTALS

1. WHEN APPLICABLE, THE CONTRACTOR SHALL SUBMIT, FOR REVIEW, DRAWINGS AND CALCULATIONS FOR ALL PERFORMANCE ASSEMBLIES / STRUCTURAL COMPONENTS IDENTIFIED IN THE GENERAL NOTES AND LISTED BELOW. THE DESIGN OF THESE ASSEMBLIES / ELEMENTS SHALL BE PERFORMED BY LICENSED ENGINEER RETAINED BY THE CONTRACTOR. ALL SUBMITTALS SHALL BE SIGNED AND SEALED BY ENGINEERS LICENSED IN THE STATE OF THE PROJECT'S JURISDICTION. REVIEW SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT REQUIREMENTS AS INDICATED ON THE DRAWINGS AND IN THE GENERAL NOTES.

   A. STEEL CONNECTIONS:
   DESIGNS SHALL INCLUDE DESIGN AND DETAILING OF ALL CONNECTIONS NOT FULLY DETAILED WITHIN THE CONTRACT DOCUMENTS. CONNECTION STANDARDS SHALL BE IN ACCORDANCE WITH TYPICAL CONNECTION DETAILS AS INDICATED ON THE DRAWINGS. CONNECTIONS SHALL INCLUDE SHEAR / MOMENT CONNECTIONS AND SHALL BE DESIGNED IN ACCORDANCE WITH THE AISC "MANUAL OF STEEL CONSTRUCTION"

   B. NON-LOAD BEARING GAUGE METAL STUD WALLS:
   DESIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES. BACK UP SYSTEM AND STUD WALLS SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF L/600 (BRICK/STONE) OF THE SPAN, OR 3/8", WHICHEVER IS LESS, & L/360 (ALL OTHER FINISHES), OR 1/2", WHICHEVER IS LESS.

   C. CURTAIN WALL SYSTEMS:
   DESIGNS SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL REQUIREMENTS BY APPLICABLE BUILDING CODES. CURTAIN WALL SHALL BE DESIGNED FOR COMPONENTS & CLADDING WIND PRESSURES WITH A MAXIMUM DEFLECTION OF L/120 OR 3/4" WHICHEVER IS LESS. CONNECTIONS TO EXISTING STRUCTURE SHALL ACCOUNT FOR VERTICAL DEFLECTION DUE TO LIVE LOAD OF BEAMS AND SLABS UP TO A MAXIMUM OF 1 1/2"

   D. METAL STAIRS, RAILINGS, GUARDRAILS, AND LADDERS:
   DESIGN SHALL TAKE INTO ACCOUNT ALL VERTICAL AND LATERAL LOADS REQUIRED BY APPLICABLE BUILDING CODES. ALL CONNECTIONS TO BUILDING STRUCTURE SHALL CONCENTRICALLY LOAD FRAMING MEMBERS WHERE EVER POSSIBLE. WHERE ECCENTRIC CONNECTIONS TO STRUCTURE ARE NECESSARY SUPPLEMENTARY FRAMING MAY BE REQUIRED TO STRENGTH MEMBERS. ANY ADDITIONAL FRAMING IDENTIFIED BY E.O.R. DURING SHOP DRAWING REVIEW SHALL BE ADDED TO STRUCTURE AT NO ADDITIONAL COST TO OWNER. HANDRAILS / GUARDRAILS SHALL BE DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF IBC SECTION 1607.8.

   E. PRE-FABRICATED CANOPIES / AWNINGS:
   DESIGN SHALL TAKE INTO ACCOUNT ALL VERTICAL LOADS INCLUDING UPLIFT REQUIRED BY APPLICABLE BUILDING CODES. ALL CONNECTIONS TO BUILDING STRUCTURE SHALL CONCENTRICALLY LOAD FRAMING MEMBERS WHERE EVER POSSIBLE. WHERE ECCENTRIC CONNECTIONS TO STRUCTURE ARE NECESSARY SUPPLEMENTARY FRAMING MAY BE REQUIRED TO STRENGTH MEMBERS. ANY ADDITIONAL FRAMING IDENTIFIED BY E.O.R. DURING SHOP DRAWING REVIEW SHALL ADDED TO STRUCTURE AT NO ADDITIONAL COST TO OWNER.

SHOP DRAWINGS

1. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL COMPONENTS. SUBMISSIONS TO THE DESIGN PROFESSIONAL SHALL INCLUDE, BUT ARE NOT LIMITED TO THE ITEMS INDICATED IN THE SHOP DRAWING SUBMITTAL SCHEDULE.
2. SHOP DRAWINGS SHALL BEAR THE CONTRACTOR'S STAMP OR APPROVAL WHICH SHALL CONSTITUTE CERTIFICATION THAT THE CONTRACTOR HAS VERIFIED ALL CONSTRUCTION CRITERIA, MATERIALS, AND SIMILAR DATA AND HAS CHECKED EACH DRAWING FOR COMPLETENESS, COORDINATION, AND COMPLIANCE WITH THE CONTRACT DOCUMENTS.
3. ANY SUBMISSION INDICATED TO INCLUDE SIGNED & SEALED CALCULATIONS AND DO NOT MEET MINIMUM DESIGN / SUBMISSION REQUIREMENTS SHALL BE IMMEDIATELY REJECTED.
4. ALL SUBMITTALS SHALL INDICATE THE ISSUE DATE OF STRUCTURAL DRAWINGS UTILIZED WHEN PREPARING SUBMITTAL. IN CONTRACT DOCUMENT DATE IS NOT CURRENT, SUBMITTAL SHALL BE IMMEDIATELY REJECTED.
5. ALL UPDATES / CHANGES TO SUBMITTALS AS PART OF RESUBMISSION SHALL BE BUBBLED. ANY RESUBMISSION WITHOUT REVISION BUBBLES SHALL BE IMMEDIATELY REJECTED.
6. IT IS THE FABRICATOR & CONTRACTOR RESPONSIBILITY TO VERIFY ALL DIMENSIONS ARE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
7. ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DOCUMENTS SHALL BE BUBBLED BY MANUFACTURER OR FABRICATOR. ANY OF THE AFOREMENTIONED WHICH ARE NOT BUBBLED OR FLAGGED BY SUBMITTING PARTIES SHALL BE CONSIDERED NOT APPROVED AFTER ENGINEERS REVIEW, UNLESS NOTED ACCORDINGLY.
8. THE ENGINEER HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO CONTRACT DOCUMENTS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW. ANY REPRODUCTION OF THE ORIGINAL STRUCTURAL DOCUMENTS ON THE SHOP DRAWINGS IS PROHIBITED AND SHALL BE IMMEDIATELY REJECTED.
9. THE SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT ARE NOT TO BE CONSIDERED CHANGES TO THE CONTRACT DOCUMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAKE SURE ITEMS ARE CONSTRUCTED TO CONTRACT DOCUMENTS.
10. THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY OTHERS RESTS WITH THE DESIGNING OR SUBMITTING PARTY.
11. REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH HE FABRICATOR / CONTRACTOR.
12. SHOP DRAWINGS FOR ALL STRUCTURAL MATERIALS TO BE SUBMITTED TO ARCHITECT FOR REVIEW PRIOR TO THE START OF FABRICATION OR COMMENCEMENT OF WORK. REVIEW PERIOD SHALL BE A MINIMUM OF TWO (2) WEEKS.
13. SUBMITTAL MARKED "NO EXCEPTIONS TAKEN" OR "MAKE CORRECTIONS NOTED" DOES NOT REQUIRE RESUBMISSION TO E.O.R. SUBMITAL MARKED "REVISE / RESUBMIT" OR "REJECTED" SHALL REQUIRE RESUBMISSION.
14. FABRICATOR / CONTRACTOR REQUEST FOR REVIEW / VERIFICATION OF PREVIOUSLY APPROVED SUBMITTAL SHALL BE AT AN ADDITIONAL COST TO THE PROJECT AND SHALL BE SUBMITTED TO OWNER AS CHANGE ORDER TO STRUCTURAL ENGINEERING CONTRACT.

SHOP DRAWING SUBMITTAL SCHEDULE

	MATERIAL/COMPONENT	SUBMITTAL REQUIRED	SIGNED AND SEALED
		YES	YES
	FOUNDATION		
1	GROUND IMPROVEMENT	--	--
2	PILES (TIMBER/STEEL/HELICAL)	--	--
	CAST-IN-PLACE CONCRETE		
1	CONCRETE MIX DESIGN	X	--
2	CONCRETE REINFORCING	X	--
3	CONSTRUCTION & EXPANSION JOINT LOCATIONS	X	--
	MASONRY		
1	MASONRY REINFORCING	X	--
2	MASONRY MATERIALS; GROUT, MORTAR, CMU BLOCK AND BOND BEAMS	X	--
3	PRECAST CONCRETE LINTEL	X	--
4	CONSTRUCTION & EXPANSION JOINT LOCATIONS	X	--
	METALS		
1	STRUCTURAL STEEL	X	--
2	STEEL CONNECTION	X	X
3	METAL DECK	X	--
4	STRUCTURAL COMPOSITE BEAM HEADED STUDS	X	--
5	STEEL BAR JOIST	X	--
6	STEEL STAIR / PLATFORMS / RAILINGS	X	X
7	STEEL GUARDS / SAFETY RAILINGS	--	--
8	STONE/BRICK LINTELS AND RELIEVING ANGLES	--	--
9	COMSLAB	--	--
10	DELTABEAM	--	--
11	COLD FORMED METAL FRAMING (LOAD BEARING)	--	--
12	COLD FORMED METAL FRAMING (NON-LOAD BEARING)	--	--
13	COLD FORMED METAL TRUSS LAYOUT & CALCULATIONS	--	--
	WOOD AND COMPOSITES		
1	WOOD LUMBER, HANGERS AND SHEATHING, HOLDDOWNS	--	--
2	WOOD TRUSS LAYOUT AND CALCULATIONS	--	--
	OTHER		
1	ELEVATOR	X	--
2	CRANE	--	--
3	CONCRETE TEMPORARY SHORING	--	--
4	POST INSTALLED ANCHORS, MATERIALS, ADHESIVES	X	--
5	FABCON PRECAST WALL PANELS	--	--

NOTES:
1. REFERENCE PERFORMANCE ASSEMBLIES NOTES FOR ADDITIONAL INFORMATION CONCERNING MINIMUM DESIGN & SUBMISSION REQUIREMENTS FOR DELEGATED DESIGN ITEMS.

ABBREVIATION KEY

ADD'L	ADDITIONAL	L.P.	LOW POINT
AESS.	ARCHITECTURALLY EXPOSED	L.W.	LONG WAY
ALT.	ALTERNATE	MAX	MAXIMUM
ARCH.	ARCHITECTURAL	M.C.	MOMENT CONNECTION
BAL.	BALANCE	M.D.	METAL DECK
BM.	BEAM	MECH.	MECHANICAL
B.O.S.	BOTTOM OF STEEL	MIN.	MINIMUM
BOT.	BOTTOM	MISC.	MISCELLANEOUS
BP.	BASE PLATE	M.O.	MASONRY OPENING
BR.	BEARING PLATE	MOM.	MOMENT
C	CAMBER	NFP.	NO FIREPROOFING
C.J.	CONTRACTION JOINT	N.L.B.	NON-LOAD BEARING WALL
CL.	CENTERLINE	N.T.S.	NOT TO SCALE
CLR.	CLEARANCE	O.C.	ON CENTER (SPACING)
COL.	COLUMN	O.D.	OUTSIDE DIAMETER
CONC.	CONCRETE	OPNG.	OPENING
CONT.	CONTINUOUS	OPP.	OPPOSITE
CONST.	CONSTRUCTION	P.A.F.	POWDER ACTUATED FASTENER
CVR.	COVER	PLT.	PLATE
DIA.	DIAMETER	PSI.	POUNDS PER SQUARE INCH
DBL.	DOUBLE	PSF.	POUNDS PER SQUARE FEET
DWG.	DRAWING	R.	RADIUS
DWL(S)	DOWEL(S)	R.A.	RIGHT END
EA.	EACH	REF.	REFERENCE
EE	EACH END	REINF.	REINFORCING
EF.	EACH FACE	REQ'D	REQUIRED
ELEV.	ELEVATION	S.C.	SHEAR CONNECTION
E.O.D.	EDGE OF DECK	SCHD.	SCHEDULE
E.O.S.	EDGE OF SLAB	SECT.	SECTION
EQ.	EQUAL SPACING	SIM.	SIMILAR.
EW	EACH WAY	S.O.G.	SLAB ON GRADE
EXP. JT.	EXPANSION JOINT	SR.	STUDRAIL
EX. OR E	EXISTING	SST.	STAINLESS STEEL
FIN.	FINISH	STIFF.	STIFFENER
F.L.	FACADE LINE	STRUCT.	STRUCTURE
FLG.	FLANGE FT FEET OR FOOT	S.W.	SHORT WAY
FLR.	FLOOR	SYMM.	SYMMETRICAL T TOP
F.P.	FIREPROOFING (FIREPROOF)	T.G.	TRANSFER GIRDER
FS.	FOOTING STEP	THK.	THICK
FTG.	FOOTING	T.O.	THROUGH OUT
Fy	YIELD STRESS	T.O.B.	TOP OF BUTTRESS
GALV.	GALVANIZED	T.O.P.	TOP OF PIER.
GC COORD	GC. COORDINATE	T.O.S.	TOP OF STEEL
GR.	GRADE	T.O.W.	TOP OF WALL
HOR.	HORIZONTAL	TYP	TYPICAL
H.P.	HIGH POINT	U.N.O.	UNLESS NOTED OTHERWISE
H.S.	HIGH STRENGTH	VA.	VARIES
H.S.B.	HIGH STRENGTH BOLT	VERT.	VERTICAL
HT.	HEIGHT	V.I.F.	VERIFY IN FIELD
IN.	OR" INCHES	W/	WITH
JST.	JOIST	W.C.	WALL COLUMN
KSI.	KIPS PER SQUARE INCHES	W.L.	WOOD LEDGER
LLH.	LONG LEG HORIZONTAL	W.P.	WORK POINT
LLV.	LONG LEG VERTICAL	W.S.	WALL STEP
LONG.	LONGITUDINAL	W.W.F.	WIRE WELDED FABRIC

SPECIAL INSPECTIONS

SPECIAL INSPECTION SHALL MEET THE REQUIREMENTS OF THE 2021 INTERNATIONAL BUILDING CODE (FLORIDA EDITION). WHERE ALL SPECIAL INSPECTOR(S) SHALL BE HIRED BY THE OWNER TO PERFORM THE REQUIRED SPECIAL INSPECTIONS, THE NAMES OF PERSONS OR FIRMS WHO ARE TO PERFORM THE SPECIAL INSPECTIONS SHALL BE FORWARDED TO THE BUILDING OFFICIAL FOR APPROVAL. THE SPECIAL INSPECTOR(S) SHALL COMPLETE AND SUBMIT ALL FORMS REQUIRED BY **OKEECHOBEE, FLORIDA**.

1. SPECIAL INSPECTOR(S) SHALL:
   A. OBSERVE THE WORK ASSIGNED FOR CONFORMANCE TO THE APPROVED DRAWING AND SPECIFICATIONS.
   B. FURNISH INSPECTION REPORTS TO THE ENGINEER OF RECORD AND BUILDING DEPARTMENT. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF NOT CORRECTED TO THE ENGINEER AND THE BUILDING DEPARTMENT.
   C. SUBMIT TO THE ENGINEER OF RECORD AND THE BUILDING DEPARTMENT A SIGNED FINAL REPORT STATING THAT THE WORK WAS IN CONFORMANCE WITH THE APPROVED DRAWINGS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC.
2. SPECIAL INSPECTION NOTES:
   A. CONTINUOUS SPECIAL INSPECTION IS ALWAYS REQUIRED DURING THE PERFORMANCE OF THE WORK UNLESS SPECIFICALLY NOTED BELOW.
   B. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE THE SPECIAL INSPECTOR(S) WITH ADVANCE NOTICE, NO LESS THAN ONE WORKING DAY, OF THE INITIATION OF ANY WORK REQUIRED TO HAVE SPECIAL INSPECTIONS. ALL WORK PERFORMED WITHOUT REQUIRED SPECIAL INSPECTION WILL BE SUBJECT TO REMOVAL.

SPECIAL INSPECTION SCHEDULE

Y	N	SPECIAL INSPECTION	CODE/SECTION
X		STEEL CONSTRUCTION	IBC 1705.2
X		STEEL CONSTRUCTION - STRUCTURAL STEEL	IBC 1705.2.1
X		STEEL CONSTRUCTION - COLD FORMED STEEL DECK	IBC 1705.2.2
X		STEEL CONSTRUCTION - OPEN-WEB JOISTS/JOIST GIRDERS	IBC 1705.2.3
	X	STEEL CONSTRUCTION - COLD-FORMED TRUSSES (L>60ft.)	IBC 1705.2.4
X		CONCRETE CONSTRUCTION	IBC 1705.3
X		CONCRETE CONSTRUCTION - WELDING (REBAR)	IBC 1705.3.1
X		CONCRETE CONSTRUCTION - MATERIAL TESTS	IBC 1705.3.2
X		MASONRY CONSTRUCTION	IBC 1705.4
	X	MASONRY CONSTRUCTION - RISK CATEGORY IV MASONRY	IBC 1705.4.1
	X	MASONRY CONSTRUCTION - VERTICAL MASONRY FOUNDATIONS	IBC 1705.4.2
X		SOILS	IBC 1705.6
	X	DRIVEN DEEP FOUNDATIONS	IBC 1705.7; 1705.10
	X	CAST-IN-PLACE DEEP FOUNDATIONS	IBC 1705.8
	X	HELICAL PILE FOUNDATIONS	IBC 1705.9
	X	FABRICATED ITEMS (SEE NOTE 2)	IBC 1705.11
	X	SEISMIC RESISTANCE (SPECIAL INSPECTIONS)	IBC 1705.13
	X	SEISMIC RESISTANCE (TESTING)	IBC 1705.14
	X	SPRAYED FIRE-RESISTANT MATERIALS	IBC 1705.15
	X	MASTIC & INTUMESCENT FIRE-RESISTANT COATINGS	IBC 1705.16
X		POST INSTALLED ANCHORS	IBC TABLE 1705.3

NOTES:
1. REFERENCE STATEMENT OF SPECIAL INSPECTIONS FOR LIST OF ALL REQUIRED INSPECTIONS LISTED ABOVE.
2. WHERE FABRICATION OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP, CONTINUOUS SPECIAL INSPECTION IS REQUIRED DURING THE PERFORMANCE OF THE WORK EXCEPT AS ALLOWED IN IBC SECTION 1704.2.5.1 AND UNLESS SPECIFICALLY NOTED BELOW.

SYMBOLS KEY

	CONCRETE WALL - LOAD BEARING (UP/DOWN)		TIMBER PILE OR STEEL PIPE PILE
	CONCRETE WALL - LOAD BEARING (TERMINATES AT UNDERSIDE OF SLAB)		STEEL H-PILE
	CMU WALL - LOAD BEARING (UP/DOWN)		BEAM TO COLUMN MOMENT CONNECTION
	CONCRETE WALL - LOAD BEARING (CHANGE IN WIDTH)		BEAM TO BEAM MOMENT CONNECTION
	CONCRETE WALL - LOAD BEARING (UP FROM FLOOR LEVEL)		TORSIONAL MOMENT CONNECTION
	LOAD BEARING STUD WALL (BELOW)		DIRECTION OF MISC. FRAMING / DECK
	SHEAR WALL (BELOW)		STEP IN SLAB/FLOOR
	LOAD BEARING CMU WALL - ABOVE (FOR REFERENCE)		SLOPE IN SLAB/FLOOR
	LOAD BEARING STUD WALL - ABOVE (FOR REFERENCE)		TWO-WAY CONCRETE SLAB
	NON-LOAD BEARING CMU WALL - ABOVE (FOR REFERENCE)		ONE-WAY CONCRETE SLAB
	CONCRETE COLUMN DOWN		CONCRETE SLAB ON GRADE
	STEEL COLUMN DOWN		CONCRETE COLUMN UP
	STEEL COLUMN UP & DOWN		STEEL COLUMN UP
	STEEL COLUMN		WOOD POST DOWN DOUBLE 2X JACK STUD U.N.O. (SEE NOTE 4)
	OPENING / PENETRATION		INDICATES POST ABOVE
		D	SLAB DEPRESSION

NOTES:
1. REFERENCE TYPICAL CONCRETE WALL CONSTRUCTION DETAIL FOR ADDITIONAL INFORMATION
2. REFERENCE TYPICAL CMU WALL CONSTRUCTION DETAIL ON FOR ADDITIONAL INFORMATION
3. REFERENCE TYPICAL INTERIOR NON-LOAD BEARING CMU CONSTRUCTION DETAIL FOR ADDITIONAL INFORMATION
4. DOUBLE 2X POST SIZE SHALL MATCH ADJACENT WALL SIZE (U.N.O.)

STRUCTURAL DRAWING LIST

● ISSUED - NEW SHEET
◒ ISSUED - REVISION MADE
○ ISSUED - NO REVISION MADE
⊗ SHEET REMOVED

			DRAWING TITLE	SHEET NO.	
		●	GENERAL NOTES & DESIGN CRITERIA	S001	1
		●	STRUCTURAL SPECIFICATIONS	S002	2
		●	STRUCTURAL SPECIFICATIONS	S003	3
		●	REBAR LAP & DEVELOPMENT SCHEDULES	S004	4
		●	SCHEDULES	S005	5
		●	FOUNDATION PLAN	S101	6
		●	CONTROL JOINT PLAN	S101A	7
		●	2ND FLOOR FRAMING PLAN	S102	8
		●	ROOF FRAMING PLAN	S103	9
		●	FOUNDATION DETAILS	S301	10
		●	FRAMING DETAILS	S301	11
		●	ROOF FRAMING DETAILS	S311	12
		●	TYPICAL SPREAD FOOTING DETAILS	S501	13
		●	TYPICAL SLAB ON GRADE DETAILS	S502	14
		●	GRADE BEAM TYPICAL DETAILS	S504	15
		●	TYPICAL ELEVATOR PIT DETAILS	S506	16
		●	TYPICAL CONCRETE WALLS DETAILS	S516	17
		●	STEEL COLUMN AND BASE PLATE DETAILS	S521	18
		●	STEEL CONNECTIONS DETAILS	S522	19
		●	STEEL BAR JOISTS TYPICAL DETAILS	S524	20
		●	COMPOSITE METAL DECK TYPICAL DETAILS	S525	21
		●	MASONRY WALL CONSTRUCTION DETAILS	S531	22
		●	MASONRY WALL LINTELS TYPICAL DETAILS	S532	23
		●	MASONRY WALL TYPICAL DETAILS	S533	24

NOTES:
1. STANDARD SHEET NUMBERING:
   S000 SERIES - GENERAL NOTES
   S100 SERIES - PLANS
   S200 SERIES - FOUNDATION DETAILS
   S300 SERIES - SUPERSTRUCTURE DETAILS
   S400 SERIES - LATERAL BRACING DETAILS
   S500 SERIES - TYPICAL DETAILS
2. S500 SERIES SHEET NUMBERING MAY NOT BE SEQUENTIAL BASED ON BUILDING MATERIALS UTILIZED WITHIN STRUCTURE.

RISE | FL architecture
ARCHITECT
NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516.249.3746
NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMON GREENBAUM, PA
SHMON@RISEARCHITECTURE.COM
908.674.8717
FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, PA
SHMON@RISEARCHITECTURE.COM
908.674.8717
STRUCTURAL CONSULTANT
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com
MEP CONSULTANT
CIVIL CONSULTANT
DESIGNER CONSULTANT

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
GENERAL NOTES & DESIGN CRITERIA

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:33:24 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted
DATE:
PROJECT NO.: 2318_07

DRAWING NO.: S001

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:15:58 AM

SUPERIMPOSED DESIGN LOAD SCHEDULE (ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)		
COMPONENT \ AREA	2ND FLOOR	ROOF
FLOOR/ROOF FINISHES	10	2
M/E/P	8	8
CEILING	1	1
PAVERS/SOLAR	-	8
FIRE PROOFING/INSULATION	1	4
MISC.	-	-
TOTAL DEAD LOAD	20	23
TOTAL LIVE LOAD	125	20

NOTES:
1. LOADS INDICATED WITHIN SCHEDULE INCLUDE SUPERIMPOSED DEAD & LIVE LOADS. (DOES NOT INCLUDE SELF-WEIGHT OF FLOOR STRUCTURE).
2. SELF-WEIGHT OF STRUCTURE ACCOUNTS FOR ALL COMPONENTS ASSOCIATED WITH FLOOR STRUCTURE INCLUDING BUT NOT LIMITED TO SLABS, SUB-FLOOR, METAL DECK, JOISTS, & BEAMS.
3. ALL LIVE LOADS LESS THAN 80psf INCLUDE 15psf FOR PARTITION LOADS.
4. STAIRS AND CORRIDORS HAVE BEEN DESIGNED FOR 100PSF LIVE LOADING.
5. DESIGN SNOW LOADING AND DRIFTS WERE DESIGNED IN ACCORDANCE WITH ASCE 7-16.

SNOW LOAD CRITERIA:
Is: IMPORTANCE FACTOR..................1.0
CE: EXPOSURE FACTOR........................1.0
CT: THERMAL FACTOR..........................1.0
PG: GROUND SNOW LOAD...................0 PSF
Pf: FLAT ROOF SNOW LOAD.............0 PSF
Cs: ROOF SLOPE FACTOR..................1.0
Ps: SLOPED ROOF SNOW LOAD.......0 PSF

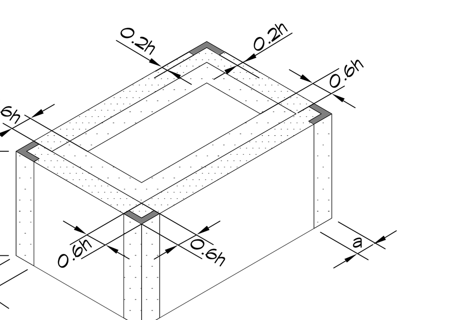
ROOF STRUCTURE DESIGNED FOR THE GREATER OF SNOW LOAD OR LIVE LOAD INDICATED ON DESIGN LOAD SCHEDULE.

LATERAL LOAD DESIGN SCHEDULE

ITEM	SYMBOL	VALUE	REFERENCE
WIND LOAD			
WIND OCCUPANCY CATEGORY	-	II	TABLE 1.5-1
BASIC WIND SPEED	V	143 mph	FIGURE 26.5-1
ASD WIND SPEED	Vasd	111 mph	
WIND EXPOSURE	-	D	SECTION 26.7.3
INTERNAL PRESSURE COEFFICIENT	Gcpi	+/- 0.18	
SEISMIC LOAD			
ITEM	SYMBOL	VALUE	REFERENCE
SEISMIC OCCUPANCY CATEGORY	-	II	TABLE 1.5-1
SEISMIC IMPORTANCE FACTOR	I	1.00	TABLE 1.5-2
SEISMIC SITE CLASS	-	E	TABLE 20.3-1
SPECTRAL RESPONSE ACCELERATION	S1	0.027 g	FIGURE 22
MAPPED SPECTRAL ACCELERATION	Ss	0.062 g	FIGURE 22
DESIGN SPECTRAL RESPONSE COEFFICIENT	SDS	0.054	SECTION 11.4.5
DESIGN SPECTRAL RESPONSE COEFFICIENT	SD1	0.038	SECTION 11.4.5
SEISMIC DESIGN CATEGORY	-	A	TABLE 11.6-1
LATERAL SYSTEM			
BASIC SEISMIC-FORCE RESISTANCE SYSTEM	-	INTERMEDIATE REINFORCED MASONRY SHEAR WALLS	TABLE 12.2-1
SEISMIC RESPONSE COEFFICIENT	Cs	0.015	SECTION 12.8.1.1
RESPONSE MODIFICATION COEFFICIENT	R	3 1/2	TABLE 12.2-1
DEFLECTION AMPLIFICATION FACTOR	Cd	2 1/4	TABLE 12.2-1
SEISMIC BASE SHEAR ANALY. PROC.	V=CsW	EQUIVALENT LATERAL FORCE METHOD	SECTION 12.8

COMPONENTS AND CLADDING WIND LOADS

(145 MPH)



ROOFS-ZONE 1' / WALLS-ZONE 4 | ROOFS-ZONE 1 / WALLS-ZONE 5 | ROOFS-ZONE 2 | ROOFS-ZONE 3

WIND LOADS ON COMPONENTS AND CLADDING

EFFECTIVE WIND AREA (SQ.FT)	WALLS: DESIGN WIND PRESSURE	WALLS SUCTION: ZONE 4	WALLS SUCTION: ZONE 5	ROOF: DESIGN WIND PRESSURE	ROOF SUCTION: ZONE 1'	ROOF SUCTION: ZONE 1	ROOF SUCTION: ZONE 2	ROOF SUCTION: ZONE 3
≤ 10	+50.9 PSF	-55.1 PSF	-67.9 PSF	+22.6 PSF	-50.9 PSF	-88.6 PSF	-116.9 PSF	-116.9 PSF
20	+48.6 PSF	-52.9 PSF	-63.3 PSF	+21.2 PSF	-50.9 PSF	-82.7 PSF	-109.4 PSF	-109.4 PSF
50	+45.7 PSF	-49.9 PSF	-57.4 PSF	+19.3 PSF	-50.9 PSF	-75.0 PSF	-99.4 PSF	-99.4 PSF
100	+43.4 PSF	-47.6 PSF	-52.9 PSF	+17.9 PSF	-50.9 PSF	-69.2 PSF	-91.9 PSF	-91.9 PSF

NOTES:
1. DISTANCE 'a'= (13 FEET)
2. 'h' INDICATES MEAN ROOF HEIGHT, IN FEET.
3. WHERE ROOF PARAPETS ARE PROVIDED THAT ARE ≥ 3'-0", ZONE 3 PRESSURES SHALL BE REDUCED TO ZONE 2 PRESSURES.
4. + AND - INDICATE PRESSURE ACTING TOWARD AND AWAY FROM THE SURFACE, RESPECTIVELY.
5. LINEAR INTERPOLATION BETWEEN TRIBUTARY AREAS IS PERMITTED.
6. HIGHER LOADS WITHIN "a" ZONE ARE APPLICABLE AT ALL SIGNIFICANT BUILDING CORNERS AND ROOF STEPS.
7. THESE LOADS ARE BASED ON ASCE 7-16. LOADS ARE LRFD ULTIMATE LOADS, MULTIPLY VALUES BY 0.6 FOR ASD SERVICE LOADS.

CAST-IN-PLACE CONCRETE TESTING/INSPECTIONS

1. EVALUATION AND ACCEPTANCE OF CONCRETE STRUCTURES SHALL BE IN ACCORDANCE WITH ACI 301.
2. CONCRETE SHALL NOT BE POURED UNTIL THE PLACEMENT OF REINFORCING HAS BEEN APPROVED BY THE INSPECTION AGENCY.
3. INSPECTIONS SHALL BE PERFORMED BY A SPECIAL INSPECTOR WHO HAS BEEN APPROVED BY THE ENGINEER OF RECORD & BUILDING OFFICIAL. THE SPECIAL INSPECTOR SHALL VERIFY THAT ALL REINFORCEMENT, TIES, ANCHORS, & SLEEVES WERE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, THE APPLICABLE ICC ESR REPORTS AND THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. THE INSPECTION SHALL INCLUDE VERIFICATION OF ANCHOR SPACING, EMBEDMENT AND EDGE DISTANCE REQUIREMENTS.
4. REINFORCING STEEL SHALL BE INSTALLED TO WITHIN THE FOLLOWING TOLERANCES PER ACI 117, "STANDARD SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS":
   CONCRETE COVER FOR SLAB TOP AND BOTTOM BARS.......................... ±1/4"
   COVER FOR OTHER REINFORCING STEEL.. ±3/8"
   SPECIFIED SPACING BETWEEN PARALLEL BARS IN SLABS........................ (SPECIFICED SPACING/4") BUT NOT TO EXCEED 1"
   HORIZONTAL DEVIATION FROM SPECIFIED LOCATION, U.N.O.................... ±3"
   SPACING AND LOCATION OF BEAM STIRRUPS.. (BEAM DEPTH IN INCHES/12) x 1"
   SPACING AND LOCATION OF COLUMN TIES.. (MIN. COL. DIM. IN INCHES/12) x 1"
   LOCATION OF ENDS OF BARS PERPENDICULAR TO SLAB EDGES.......... ±1"

   THE ABOVE LIST OF PERMITTED TOLERANCES MUST BE PROVIDED ON ALL REINFORCING STEEL PLACING DRAWINGS. PLACING DRAWINGS THAT DO NOT PROVIDE THIS LIST OF TOLERANCES WILL BE REJECTED.
5. THE CONCRETE SUPPLIER SHALL SUBMIT MIX DESIGNS FOR REVIEW. COMPRESSIVE STRENGTH MUST BE SUBSTANTIATED BY A SUITABLE EXPERIENCE RECORD OR BY THE METHOD OF LABORATORY TRIAL BATCHES. THE PERTINENT CRITERIA OF ACI 318 SHALL APPLY TO THE PROPORTIONING OF MIX DESIGNS AND TO THE ACCEPTANCE OF CONCRETE PRODUCED FOR THE JOB. IF DURING CONSTRUCTION ANY CLASS CONCRETE FAILS TO MEET THE ACCEPTANCE CRITERIA, THE CONTRACTOR SHALL TAKE SUCH STEPS AS ARE DEEMED NECESSARY BY THE STRUCTURAL ENGINEER TO IMPROVE SUBSEQUENT TEST RESULTS AT NO ADDITIONAL COST TO THE OWNER. THE CONTRACTOR SHALL ALSO BEAR THE COST OF SPECIAL INVESTIGATION, TESTING, OR REMEDIAL WORK NECESSARY BECAUSE OF EVIDENCE OF LOW STRENGTH OR NON-CONFORMING CONCRETE OR WORKMANSHIP.
6. PREPARE A MINIMUM OF (1) CONCRETE TEST CYLINDERS AT 7 DAYS / (3) CONCRETE TEST CYLINDERS AT 28 DAYS / (1) CONCRETE TEST CYLINDER AT 56 DAYS PER BATCH OF CONCRETE. CYLINDERS SHALL BE PROPERLY CURED AND STORED. SAMPLE FRESH CONCRETE IN ACCORDANCE WITH ASTM C172.
7. RETAIN LABORATORY TO PROVIDE TESTING SERVICE. SLUMP PER ASTM C143L AIR CONTENT PER ASTM C231 OR C173, CYLINDER TESTS PER ASTM C31 AND C39. A MINIMUM OF ONE SET OF FIVE CYLINDERS SHALL BE TESTED PER: DAY, 150 CUBIC YARDS OF CONCRETE, AND 5000 SQUARE FEET OF SURFACE AREA OF SLABS AND WALLS. A MINIMUM OF (5) STRENGTH TESTS AT 28 DAYS PER CONCRETE MIXTURE MUST BE PERFORMED. SAMPLES FOR TESTS ARE TO BE TAKEN RANDOMLY. REPORTS OF ALL TESTS TO BE SUBMITTED TO THE ENGINEER OF RECORD.
8. SLUMP TESTS SHALL BE MADE PRIOR TO THE ADDITION OF PLASTICIZERS. CONCRETE FOR THE PREPARATION OF TEST CYLINDERS SHALL BE TAKEN FROM THE HOSE END FOR CONCRETE PLACED BY PUMP. PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP OF 3-1/2IN. ± 1 IN. AT THE POINT OF PLACEMENT. CONCRETE CONTAINING HIGH-RANGE WATER REDUCERS (HRWR) SHALL HAVE A SLUMP OF 4 IN. TO 8 IN.
9. ALL CONCRETE SHALL BE PLACED IN ACCORDANCE WITH ACI 117 "STANDARD SPECIFICATIONS FOR TOLERANCE FOR CONCRETE CONSTRUCTION AND MATERIALS" AND SHALL MEET THE FOLLOWING REQUIREMENTS:

   MINIMUM SLAB CONSTRUCTION TOLERANCE SPECIFICATIONS:

   - FLOOR FLATNESS (FF) = 32 OR GREATER +0" / -3/16 FOR EVERY 24"
   - FLOOR LEVELNESS (FL) = 30 OR GREATER (PRIOR TO REMOVAL OF FORMWORK)
   - ELEVATION ENVELOPE = +/-3/4" (FROM AVERAGE SLAB ELEVATION)
   - SLAB THICKNESS TOLERANCE = +/-3/8 IN. AND -1/4 IN. (FOR SLABS 12" THICK OR LESS)
   - FORMED SURFACE TOLERANCE = +/- 1/4"

   THE CONTRACTOR SHALL ALSO COORDINATE CONCRETE CLASS OF SURFACE WITH THE PROPOSED ARCHITECTURAL FINISHES.

   - CLASS A - SURFACE PROMINENTLY EXPOSED TO PUBLIC VIEW WHERE APPEARANCE IS OF SPECIAL IMPORTANCE.
   - CLASS B - COARSE-TEXTURED CONCRETE-FORMED SURFACE INTENDED TO RECEIVE PASTER, STUCCO OR WAINSCOTING.
   - CLASS C - STANDARD FOR EXPOSED SURFACE WHERE FINISHES ARE NOT SPECIFIED.
   - CLASS D - MINIMUM QUALITY OF SURFACE WHERE ROUGHNESS IS NOT OBJECTIONABLE.

CONSTRUCTION JOINTS

1. CONSTRUCTION JOINTS FOR SLABS ON METAL DECK SHALL BE LOCATED MIDWAY BETWEEN BEAMS WHERE THE JOINT IS PARALLEL TO THE BEAM SPAN. JOINTS SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF SPAN WHERE THE JOINT IS PERPENDICULAR TO THE BEAM SPAN. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS, UNLESS OTHERWISE SHOWN. ALL REINFORCING IS TO BE CONTINUOUS THROUGH JOINTS.
2. HORIZONTAL JOINTING WILL NOT BE PERMITTED IN CONCRETE CONSTRUCTION EXCEPT AS SHOWN ON THE CONTRACT DOCUMENT. VERTICAL JOINTS SHALL OCCUR AT CENTER OF SPANS AT LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER.
3. CONSTRUCTION JOINTS BETWEEN FOOTINGS AND PILASTERS AND SIMILAR JOINTS SHALL BE PREPARED BY ROUGHENING THE CONTACT SURFACE IN AN APPROVED MANNER TO A FULL AMPLITUDE OF APPROXIMATELY 14 INCHES, LEAVING THE CONTACT SURFACE FREE AND CLEAR OF LAITANCE. REINFORCED (DOWELLED) JOINTS SHALL HAVE BINDER ADDITIVE APPLIED PRIOR TO POUR.
4. PROVIDE CONTINUOUS WATERSTOPS AT ALL HORIZONTAL AND VERTICAL CONSTRUCTION JOINTS AND/OR AS NOTED ON PLAN, SIZED AND CONFIGURED TO SUIT JOINTS. MINIMUM PROVIDED WATERSTOP SHALL BE 6" PVC RIBBED WITH CENTER BULB WATERSTOP BY "GREENSTREAK" OR APPROVED EQUAL. INSTALL TO FORM CONTINUOUS, WATERTIGHT DAM, WITH FIELD JOINTS FABRICATED IN STRICT ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS.
5. CONSTRUCTION JOINTS FOR MILD-REINFORCED CONCRETE SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF SPAN. PROPOSED CONSTRUCTION JOINT LOCATIONS SHALL BE SHOWN ON REINFORCING STEEL SHOP DRAWINGS. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULKHEADS AND HORIZONTAL KEYS, UNLESS OTHERWISE SHOWN. ALL REINFORCING IS TO BE CONTINUOUS THROUGH JOINTS.

CAST-IN-PLACE ANCHORS

1. ALL ANCHORS SHALL ASSUME THE CRACKED CONCRETE DESIGN CONDITION, U.N.O.
2. THE CONTRACTOR SHALL ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ON SITE INSTALLATION TRAINING FOR EACH SPECIFIED ANCHOR TYPE. THE STRUCTURAL ENGINEER OF RECORD SHALL RECEIVE DOCUMENTATION VERIFYING THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS HAVE BEEN TRAINED PRIOR TO COMMENCEMENT OF INSTALLING ANCHORS.
3. SURVEY ANCHOR BOLTS FOR PLACEMENT AND ALIGNMENT PRIOR TO CASTING CONCRETE.
4. ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ANCHORS AND PROXIMITY OF ANCHORS TO EDGES OF CONCRETE OR MASONRY. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
5. ALL INSERTS AND SLEEVES SHALL BE CAST-IN-PLACE WHENEVER FEASIBLE. DRILLED OR POWDER DRIVEN FASTENERS WILL BE PERMITTED WHEN PROVEN TO THE SATISFACTION OF THE ENGINEER OF RECORD THAT THE FASTENERS WILL NOT SPALL THE CONCRETE AND HAVE THE SAME CAPACITY AS CAST-IN-PLACE INSERTS. WHEN INSTALLING EXPANSION BOLTS OR ADHESIVE ANCHORS, THE CONTRACTOR SHALL TAKE MEASURES TO AVOID DRILLING OR CUTTING OF ANY EXISTING REINFORCING AND DESTRUCTION OF CONCRETE. HOLES SHALL BE BLOWN CLEAN PRIOR TO PLACING BOLTS OR ADHESIVE ANCHORS.

REBAR/SPLICING

1. PROVIDE TWO (2) #3 X 4'0" AT ALL RE-ENTRANT CORNERS, PLACED ON THE DIAGONAL WITH 1 1/2" CLEARANCE FROM THE CORNER AND TOP OF SLAB. REFER TO DETAIL.
2. SPLICES IN REINFORCING STEEL SHALL BE MADE ONLY AT THOSE LOCATIONS WHERE SPLICES ARE SHOWN ON THE STRUCTURAL DRAWINGS AND AT THOSE LOCATIONS WHERE SPLICES HAVE BEEN DETAILED ON THE REINFORCING STEEL PLACEMENT DRAWINGS THAT HAVE BEEN REVIEWED AND APPROVED BY THE ENGINEER OF RECORD. ALL SPLICES SHALL BE CLASS "B" TENSION LAP SPLICES (SEE TENSION DEVELOPMENT LENGTH TABLE IN TYPICAL CONCRETE DETAILS), EXCEPT WHERE INDICATED OTHERWISE ON THE STRUCTURAL DRAWINGS. MECHANICAL SPLICE COUPLERS CAPABLE OF DEVELOPING 125% OF THE YIELD STRENGTH OF THE REINFORCING STEEL MAY BE USED INSTEAD OF TENSION LAP SPLICES AT THE CONTRACTOR'S OPTION AT ANY LOCATION. COMPRESSION LAP SPLICES (SEE COMPRESSION DEVELOPMENT AND LAP SPLICE LENGTHS TABLE IN TYPICAL CONCRETE DETAILS) MAY BE USED ONLY AT THOSE LOCATIONS WHERE SUCH SPLICES ARE SPECIFICALLY INDICATED. STAGGER SPLICES WHERE REQUIRED TO PROVIDE 1 1/2" CLEAR SPACING BETWEEN REINFORCING STEEL AT SPLICE LOCATIONS.
3. WHEREVER POSSIBLE, SPLICES OF MILD STEEL SHALL BE MADE IN A COMPRESSION AREA. NO MORE THAN 50% OF BARS (ALTERNATED) SHALL BE SPLICED IN A TENSION AREA.
4. VERTICAL DOWELS PROJECTING OUT OF FOUNDATIONS INTO WALLS SHALL BE THE SAME SIZE AND SPACING AS THE VERTICAL REINFORCING STEEL IN THE WALL, U.N.O. AND SHALL BE INSTALLED FOR THE FULL DEPTH OF THE FOUNDATION AND TERMINATE WITH STANDARD 90 DEGREE HOOKS, U.N.O.; VERTICAL DOWELS PROJECTING OUT OF THE TOPS OF WALLS INTO SLABS SHALL BE THE SAME SIZE AND SPACING AS THE VERTICAL STEEL IN THE WALLS, U.N.O. AND SHALL EXTEND TO THE TOP OF THE SLAB AND TERMINATE WITH STANDARD 90 DEGREE HOOKS, U.N.O.; ALL DOWELS SHALL BE SPLICED WITH CLASS "B" TENSION LAP SPLICES TO THE VERTICAL REINFORCING STEEL IN THE WALLS.
5. ALL HOOKS IN REINFORCING STEEL SHALL BE STANDARD 90 DEGREE HOOKS, U.N.O.
6. ALL HOOKS ON #5 AND SMALLER TOP REINFORCING STEEL IN SLABS SHALL BE STANDARD 90 DEGREE STIRRUP HOOKS; A CONTINUOUS PERPENDICULAR TOP BAR (#4 MINIMUM) SHALL BE INSTALLED INSIDE THE CORNERS OF ALL HOOKED BARS.
7. VERTICAL REINFORCING STEEL IN CONCRETE AND MASONRY WALLS WITH ONE LAYER OF REINFORCING BARS SHALL BE INSTALLED IN THE CENTER OF THE WALL, U.N.O.

CAST-IN-PLACE CONCRETE MISC. DIRECTIONS

1. MIXING, TRANSPORTING AND PLACING OF CONCRETE SHALL CONFORM TO ACI 301.
2. REFER TO THE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, LOCATIONS AND DETAILS OF ALL ARCHITECTURAL FEATURES IN THE CONCRETE; REFER TO THE ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR REQUIREMENTS FOR ALL CONCRETE FINISHES; REFER TO THE ARCHITECTURAL DRAWINGS FOR TOP OF WALL ELEVATIONS FOR ALL WALLS WHERE TOP OF WALL ELEVATIONS ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS.
3. THE CONTRACTOR IS RESPONSIBLE FOR DESIGN, ENGINEERING, AND CONSTRUCTION OF FORMWORK, CAPABLE OF SUPPORTING ALL APPLIED LOADS UNTIL THE CONCRETE IS ADEQUATELY CURED, WITHIN ALLOWABLE TOLERANCES AND DEFLECTION LIMITS.
4. ALL REINFORCEMENT SHALL BE SECURELY HELD IN PLACE WHILE PLACING CONCRETE. ADDITIONAL BARS, STIRRUPS OR CHAIRS SHALL BE PROVIDED BY THE CONTRACTOR TO FURNISH SUPPORT AND PROVIDE MINIMUM REINFORCEMENT COVER FOR ALL BARS.
5. BONDING AGENT SHALL BE USED WHERE NEW CONCRETE IS PLACED AGAINST EXISTING CONCRETE.
6. ALL BEAMS, SPANDRELS AND SLABS ARE TO BE PLACED MONOLITHICALLY UNLESS OTHERWISE SHOWN.
7. WHERE MASONRY ABUTS CONCRETE WALLS, PROVIDE DOVETAIL SLOTS AND MASONRY ANCHORS.
8. THE CONCRETE SLABS SHALL BE FINISHED FLAT AND LEVEL WITHIN TOLERANCE, TO THE ELEVATION INDICATED ON THE DRAWINGS. THE CONTRACTOR SHALL PROVIDE ADDITIONAL CONCRETE REQUIRED DUE TO FORMWORK, METAL DECK, AND FRAMING DEFLECTION TO ACHIEVE THIS FINISHED TOP OF SLAB ELEVATION. THE CONTRACTOR SHALL PROVIDE FOR A MINIMUM OF 5/8" AVERAGE THICKNESS FOR ADDITIONAL CONCRETE DURING PLACEMENT FOR ALL SLABS SUPPORTED AND FORMED ON STEEL DECK OVER THE ENTIRE FLOOR AREA. THE CONTRACTOR SHALL PROVIDE THE MEANS BY WHICH THE MAXIMUM AND MINIMUM CONCRETE SLAB THICKNESS CAN BE MONITORED AND VERIFIED DURING AND AFTER THE PLACING AND FINISHING OPERATIONS.
9. REPAIR CONCRETE EXHIBITING VOIDS DUE TO SNAP TIES, "HONEYCOMBS," ROCK POCKETS, AND RUNS, SPALLS OR OTHERWISE DAMAGED SURFACES WITH DRY PACK OR CEMENT GROUT, AND FINISH FLUSH WITH ADJOINING SURFACES. AT THE DISCRETION OF THE STRUCTURAL ENGINEER OR AS QUALIFIED BY LAB TESTING, EXCESSIVE HONEYCOMBS OR EXPOSED REINFORCEMENT THAT JEOPARDIZE THE DESIGN, SHALL BE REMOVED AND REPLACED AT THE EXPENSE OF THE CONTRACTOR.
10. ALL EXPOSED CORNERS SHALL BE CHAMFERED 3/4" UNLESS OTHERWISE INDICATED.
11. CONTRACTOR SHALL TAKE EVERY PRECAUTION TO PROTECT FINISHED SURFACES FROM STAINS OR ABRASIONS. NO FIRE SHALL BE ALLOWED IN DIRECT CONTACT WITH CONCRETE. PROVIDE ADEQUATE PROTECTION AGAINST INJURIOUS ACTION BY SUN OR WIND. FRESH CONCRETE SHALL BE THOROUGHLY PROTECTED FROM HEAVY RAIN, FLOWING WATER, AND MECHANICAL INJURY.
12. TOPS OF FOUNDATIONS SHALL BE TROWEL FINISHED AND SMOOTH.
13. PROVIDE 10 MIL. VAPOR BARRIER (SLAB) & WATERPROOFING MEMBRANE (ELEVATOR PIT) IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS. REFERENCE ARCHITECTURAL DRAWINGS FOR SPECIFICATIONS FOR WATERPROOFING MEMBRANE.
14. WATER SHALL NOT BE ADDED TO THE CONCRETE AT THE JOBSITE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE THE REQUIREMENTS OF THE CONCRETE SUPPLIER AND PUMPER TO ENSURE PUMPABLE AND WORKABLE MIX WITHOUT THE ADDITION OF WATER AT THE JOBSITE. THE USE OF PLASTICIZERS, RETARDANTS AND OTHER ADDITIVES SHALL BE AT THE OPTION OF THE CONTRACTOR SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER. FOLLOW THE RECOMMENDATIONS OF THE MANUFACTURER FOR PROPER USE OF RETARDANTS AND OTHER ADDITIVES. USE OF CALCIUM E CHLORIDE OR OTHER CHLORIDE BEARING SALTS SHALL NOT BE PERMITTED.
15. PLACE CONCRETE IN A MANNER SO AS TO PREVENT SEGREGATION OF THE MIX. DELAY FLOATING AND TROWELING OPERATIONS UNTIL THE CONCRETE HAS LOST SURFACE WATER SHEEN OR ALL FREE SLAB SURFACE. FINISHING OF SLAB SURFACES SHALL COMPLY WITH ACI RECOMMENDATIONS 302 AND 304 FOR GARAGES.
16. CONTRACTOR SHALL PROTECT CONCRETE THAT IS NOT AIR ENTRAINED BUT WHICH IS EXPOSED TO WEATHER DURING CONSTRUCTION FROM FREEZE THAW DAMAGE UNTIL SUCH TIME AS THE CONDITIONS IS NO LONGER EXPOSED TO FREEZE/THAW CONDITIONS.
17. FOUNDATION SURFACES AGAINST WHICH CONCRETE IS TO BE PLACED MUST BE FREE FROM STANDING WATER, MUD AND DEBRIS. SURFACES SHALL BE CLEAN AND FREE FROM OIL, OBJECTIONABLE COATINGS, AND LOOSE OR UNSOUND MATERIAL.
18. PROTECT CONCRETE FROM SUN AND RAIN. DO NOT PERMIT CONCRETE TO BECOME DRY DURING CURING PERIOD. CONCRETE SHALL NOT BE SUBJECTED TO ANY LOADS UNTIL CONCRETE IS COMPLETELY CURED, AND UNTIL CONCRETE HAS ATTAINED ITS 28 DAY STRENGTH AND 14 DAYS MINIMUM.
19. UPON COMPLETION OF FINISHING OPERATION, THE SURFACE OF SLABS SHALL BE SEALED AGAINST MOISTURE LOSS FOR 7 DAYS BY THE APPLICATION OF A CURING MEMBRANE OR BLANKET.
20. CONCRETE IN FORMS SHALL BE KEPT MOIST UNTIL REMOVAL. IMMEDIATELY UPON REMOVAL OF FORMS, AN APPROVED SPRAYED-ON CURING COMPOUND SHALL BE APPLIED TO THE CONCRETE SURFACES IN STRICT COMPLIANCE WITH THE MANUFACTURERS RECOMMENDATIONS. CURING SHALL BE MAINTAINED FOR 7 DAYS.
21. PROVIDE 7 DAY CURING IMMEDIATELY AFTER FINISHING USING ONE OF THE FOLLOWING METHODS:
    A. CONTINUOUSLY WATERED BURLAP
    B. WATERPROOF MEMBRANES
    C. SPRAYED-ON LIQUID MEMBRANE

    REFER TO THE MANUFACTURER'S SPECIFICATIONS FOR REQUIREMENTS. PROTECT THE CONCRETE SURFACE BETWEEN FINISHING OPERATIONS ON HOT, DRY DAYS OR ANY TIME PLASTIC SHRINKAGE CRACKS DEVELOP USING WET BURLAP, PLASTIC MEMBRANES OR FOGGING. PROTECT CONCRETE DECK AT ALL TIMES FROM RAIN, HAIL OR OTHER INJURIOUS EFFECTS.
22. PROVIDE POUR STOP MATERIAL WHERE NOT INDICATED ON PLAN AS REQUIRED TO COMPLETE JOB.
23. HOT WEATHER CONCRETING (ABOVE 90°F):
    WHEN PLACING CONCRETE IN HOT WEATHER CONDITIONS THAT COULD ADVERSELY AFFECT THE PROPERTIES AND SERVICEABILITY OF CONCRETE, PREPARATIONS AND PROCEDURES OUTLINED IN ACI 305R SHOULD BE FOLLOWED UNLESS OTHERWISE NOTED IN CONSTRUCTION SPECIFICATIONS.
24. COLD WEATHER CONCRETING (BELOW 40°F):
    WHEN PLACING CONCRETE IN COLD WEATHER CONDITIONS THAT COULD ADVERSELY AFFECT THE PROPERTIES AND SERVICEABILITY OF CONCRETE, PREPARATIONS AND PROCEDURES OUTLINED IN ACI 306R SHOULD BE FOLLOWED UNLESS OTHERWISE NOTED IN CONSTRUCTION SPECIFICATIONS.
25. MASS CONCRETE POUR:
    ANY MASS CONCRETE AS DEFINED BY ACI 301 SHALL BE SUBJECT TO CONSTRUCTION REQUIREMENTS OF ACI 207.

SPREAD FOOTING FOUNDATIONS & SUB-GRADE

1. FOUNDATIONS HAVE BEEN DESIGNED AND FOOTING ELEVATIONS ESTABLISHED ON THE BASIS OF A SUBSURFACE INVESTIGATION REPORT AND RECOMMENDATIONS PREPARED BY ATLAS TECHNICAL CONSULTANTS, LLC DATED OCTOBER 7, 2021. SEE THE REPORT FOR ADDITIONAL REQUIREMENTS. THE REQUIREMENTS CONTAINED IN THE GEOTECHNICAL REPORT ARE PART OF THE CONSTRUCTION DOCUMENTS.
2. THE FOUNDATION FOR THE STRUCTURE HAS BEEN DESIGNED FOR THE FOLLOWING ALLOWABLE SOIL BEARING PRESSURES AT A BEARING DEPTH OF APPROXIMATELY 18" BELOW FINISHED FLOOR.

   TOTAL LOAD..........................2,500 PSF (NET)
3. THE FOOTING LEVEL SHALL BE TESTED USING DROP-BAR PERCUSSION TEST OR PENETROMETER TO A DEPTH OF 3 OR 4 FEET BELOW BEARING LEVELS TO INSURE ADEQUATE BEARING MATERIALS COMPLY WITH BORING LOGS AND DESIGN CRITERIA.
4. THE BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF ONE AND HALF (1.5) FEET BELOW FINISHED GRADE, OR AS REQUIRED BY LOCAL BUILDING CODES.
5. EXCAVATION SHALL BE PERFORMED SO AS NOT TO DISTURB EXISTING ADJACENT BUILDINGS, STREETS, AND UTILITY LINES. VERIFY LOCATION OF ALL UTILITIES PRIOR TO COMMENCEMENT OF WORK. HAND EXCAVATE AROUND UTILITIES AS REQUIRED.
6. REMOVE EXISTING VEGETATION, TOPSOIL, AND UNSATISFACTORY SOILS MATERIALS. PROOF ROLL SUBGRADE TO OBTAIN UNIFORMLY DENSIFIED SUBSTRATA PRIOR TO PLACING FILL MATERIAL EVENLY IN 8" THICK (MAXIMUM) LAYERS AND COMPACTING TO REQUIRED DENSITY.
7. SEE THE GEOTECHNICAL REPORT FOR EXCAVATION, BACKFILL AND PREPARATION OF THE FOUNDATION AND SLAB-ON-GRADE SUBGRADE INCLUDING COMPACTION REQUIREMENTS. IF GEOTECHNICAL REPORT IS NOT AVAILABLE, GEOTECHNICAL ENGINEER SHALL BE CONTRACTED WITH TO PROVIDE SPECIFICATIONS.
8. THE OWNER SHALL RETAIN THE SERVICES OF A PROFESSIONAL GEOTECHNICAL ENGINEER, SUBJECT TO THE APPROVAL OF THE ARCHITECT/ENGINEER, TO PERFORM SOIL TESTING AND INSPECTION. THE GEOTECHNICAL ENGINEER SHALL INSPECT THE SUBGRADE TO VERIFY BEARING LEVELS AND ENSURE THAT THE SAFE BEARING CAPACITY MEETS OR EXCEEDS THE DESIGN VALUE INDICATED ON CONTRACT DOCUMENTS. FIELD REPORTS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER OUTLINING THE WORK PERFORMED AND TEST RESULTS.
9. THE INSPECTION AND TESTING OF ALL SUBGRADE AND COMPACTED EARTHWORK SHALL BE CONDUCTED UNDER THE SUPERVISION OF A QUALIFIED GEOTECHNICAL CONSULTANT. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER 24 HOURS PRIOR TO PLACEMENT OF CONCRETE IN THE FOOTINGS. IF UNSUITABLE SUBGRADE SOILS ARE ENCOUNTERED, THE CONTRACTOR SHALL SUBMIT RECOMMENDATIONS PREPARED BY A GEOTECHNICAL CONSULTANT TO THE STRUCTURAL ENGINEER FOR APPROVAL.
10. THE SUBGRADE AND EACH LAYER OF FILL OR BACKFILL SHALL BE COMPACTED TO A DRY DENSITY AT LEAST EQUAL TO 95% OF THE MAXIMUM DRY DENSITY ATTAINED BY THE MODIFIED PROCTOR TEST ASTM D1557.
11. IF CONDITIONS PROVE TO BE UNACCEPTABLE AT THE BEARING ELEVATIONS SHOWN, THE FOOTING BEARING ELEVATIONS MAY NEED TO BE LOWERED BASED ON THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER. FINAL BEARING ELEVATIONS AND BACKFILL RECOMMENDATIONS MUST BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO FIELD MODIFICATION. CONCRETE FOR FOUNDATIONS SHALL BE POURED ON THE SAME DAY THE SUBGRADE IS APPROVED BY THE GEOTECHNICAL ENGINEER.
12. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF GROUNDWATER ELEVATION PRIOR TO THE START OF CONSTRUCTION. LICENSED ENGINEER WITHIN THE PROJECT JURISDICTION SHALL BE CONTRACTED TO CONFIRM CURRNT WATER TABLE ELEVATION, LOCATION OF MODELING WITHIN SOIL PROFILE, IDENTIFICATION OF PERCHED WATER TABLE AND OTHER FACTORS CONSIDERED IMPORTANT TO IDENTIFYING THE ANTICIPATED MEAN HIGH WATER TABLE ELEVATION. GROUNDWATER ELEVATION MAY FLUCTUATE IN LEVEL DUE TO VARIATIONS IN THE SEASON, RAINFALL, SNOW MELT, SURFACE INFILTRATION, TEMPERATURE, CONSTRUCTION ACTIVITIES PUMPING OF DEWATERING SYSTEMS, LEAKAGE OF UTILITIES AND OTHER FACTORS. EOR/AOR SHALL BE NOTIFIED IN WRITING OF CURRENT WATER TABLE ELEVATION PRIOR TO START OF FOUNDATION CONSTRUCTION AND WE RESERVE THE RIGHT TO RECOMMEND MODIFICATION TO CONCRETE SLABS, FOUNDATION WALLS, WATERPROOFING AND SUMP PUMP SYSTEMS AS REQUIRED TO PERMIT CONSTRUCTION WHERE HIGH WATER TABLE ELEVATIONS ARE PRESENT.
13. CONCRETE FOUNDATIONS SHALL NOT BE PLACED IN WATER OR ON FROZEN EARTH.
14. UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEER'S APPROVAL.
15. SLAB-ON-GRADE HAS BEEN DESIGNED USING A SUBGRADE MODULUS OF K=100 PCI AND DESIGN LOADING OF 100 PSF. THE GEOTECHNICAL ENGINEER SHALL ENSURE SUB-GRADE IS PREPARED TO MINIMUM SPECIFICATION, OTHERWISE THE DESIGNER IS NOT RESPONSIBLE FOR DIFFERENTIAL SETTLEMENT, SLAB CRACKING OR OTHER FUTURE DEFECTS RESULTING FROM UNREPORTEDCONDITIONS MITIGATING THE ABOVE ASSUMPTIONS.
16. SLAB-ON-GRADE SHALL BE UNDERLAIN BY A MINIMUM OF SIX INCHES OF STABLE GRANULAR MATERIAL AND 10 MIL VAPOR BARRIER.
17. THE CONTRACTOR SHALL OBSERVE WATER CONDITIONS AT THE SITE AND TAKE THE NECESSARY PRECAUTIONS TO ENSURE THAT THE FOUNDATION EXCAVATIONS REMAIN DRY DURING CONSTRUCTION.
18. DEWATERING OF THE SITE DURING CONSTRUCTION IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. ANY SHEETING OR SHORING REQUIRED FOR DEWATERING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. PRECAUTIONS SHALL BE TAKEN BY THE CONTRACTOR NOT TO UNDERMINE EXISTING FOUNDATIONS. METHOD OF DEWATERING AND CALCULATIONS FOR THE APPROPRIATE SYSTEM ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
19. PROVIDE A CONTINUOUS WATERSTOP AT ALL HORIZONTAL AND VERTICAL CONSTRUCTION JOINTS IN THE ELEVATOR PIT AND ALL OTHER PIT WALLS. REFERENCE CONCRETE NOTES FOR WATERSTOP SPECIFICATIONS.
20. BACKFILL SHALL BE BROUGHT UP SIMULTANEOUSLY ON EACH SIDE OF WALLS AND GRADE BEAMS WITH A GRADE DIFFERENCE NOT TO EXCEED 2'-0" AT ANY TIME.
21. BACK-FILLING AGAINST FOUNDATION WALLS WHICH RETAIN EARTH SHALL BE DONE CAREFULLY WITH SMALL COMPACTION EQUIPMENT, AFTER CONCRETE SLAB AND ELEVATED FLOOR SLAB ARE IN PLACE AND CONCRETE HAS OBTAINED THE SPECIFIED 28-DAY COMPRESSIVE STRENGTH. TRUCKS BULLDOZERS, ETC., SHALL NOT BE ALLOWED CLOSER THAN 6'-0" TO ANY FOUNDATION WALL. CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE TEMPORARY SHORING WHERE REQUIRED.
22. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE NEED TO USE FOUNDATION REBAR AS A GROUNDING ELECTRODE SYSTEM AND SHALL BE RESPONSIBLE FOR INSTALLING THE BONDING CLAMP PRIOR TO PLACEMENT OF THE CONCRETE AS PER NJUCC BULLETIN NO. 02-2.

CAST-IN-PLACE CONCRETE SPECIFICATIONS

1. CONCRETE SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318), AND CONSTRUCTED IN ACCORDANCE WITH THE CRSI MANUAL OF STANDARD PRACTICE.
2. CONCRETE SHALL HAVE NATURAL SAND FINE AGGREGATE AND NORMAL WEIGHT COARSE AGGREGATES CONFORMING TO ASTM C33, TYPE 1 PORTLAND CEMENT CONFORMING TO ASTM C150.
3. CONCRETE IN THE FOLLOWING AREAS SHALL HAVE THE FOLLOWING MAXIMUM WATER/CEEMENT RATIO (W/CM):

	W/CM
FTG/GBS (F1 EXPOSURE)	0.55
FNDN WALLS/PIERS (F1 EXPOSURE)	0.55
INTERIOR SOG (F1 EXPOSURE)	0.55
EXTERIOR SOG (F3 EXPOSURE)	0.4
COLUMNS (F1 EXPOSURE)	0.55
STRUCTURAL SLAB (INT) (FO EXPOSURE)	0.5
STRUCT SLAB (EXT) (F2 EXPOSURE)	0.4
SLAB ON METAL DECK (FO EXPOSURE)	0.5

4. AIR ENTRAINMENT SHALL BE A MINIMUM OF 6% IN ALL EXPOSED CONCRETE.
5. MAXIMUM AGGREGATE SIZE SHALL BE:
   FOOTINGS...1-1/2"
   WALLS / GRADE BEAMS / SLAB..............3/4"
6. NORMAL WEIGHT CONCRETE (145 PCF ± 5) SHALL BE PROVIDED WITH ALL CEMENT CONFORMING TO ASTM C150, TYPE 1, WHERE NOTED, LIGHTWEIGHT SLAB CONCRETE (110 PCF ± 5) SHALL BE PROVIDED WITH ALL CEMENT CONFORMING TO ASTM C330, TYPE 1. REFERENCE PLANS AND SCHEDULE FOR THE MINIMUM CONCRETE STRENGTH AT 28 DAYS (f'c).
7. CONCRETE REINFORCEMENT BARS SHALL CONFORM TO ASTM A615, GRADE 60.
8. WHERE NOTED ON PLAN, EPOXY COATED REINFORCING STEEL SHALL CONFORM TO ASTM A775.
9. REINFORCEMENT BARS SHALL NOT BE TACK WELDED, WELDED, HEATED OR CUT UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR APPROVED BY THE STRUCTURAL ENGINEER, WELDING OF REINFORCEMENT BARS, WHEN APPROVED BY THE STRUCTURAL ENGINEER, SHALL CONFORM TO THE AMERICAN WELDING SOCIETY STANDARD D1.4. ELECTRODES FOR SHOP AND FIELD WELDING OF REINFORCEMENT BARS SHALL BE CLASS E90XX.
10. WELDED WIRE FABRIC WHEN USED SHALL CONFORM TO ASTM A185. FABRIC SHALL BE SUPPLIED IN FLAT SHEETS. FABRIC SHALL BE LAPPED WITH MINIMUM TWO MESHES AT SPLICES. WELDED WIRE FABRIC SHALL BE LOCATED NO MORE THAN 1" FROM TOP OF SLAB.
11. FIBROUS REINFORCEMENT FOR SLABS SHALL BE FIBRILLATED POLYPROPYLENE FIBERS ENGINEERED AND DESIGNED FOR USE IN CONCRETE COMPLYING WITH ASTM C 1116 TYPE III, 1/2" TO I 1/2". UNIFORMLY DISPERSE FIBERS IN THE CONCRETE MIX AT THE MANUFACTURER'S RECOMMENDED RATE BUT NOT LESS THAN 1.5 POUNDS PER CUBIC YARD.
12. GROUT SHALL BE NON-SHRINK GROUT CONFORMING TO ASTM C827, AND SHALL HAVE SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5,000 PSI. PRE-GROUTING OF BASE PLATES IS NOT PERMITTED.
13. RIGID INSULATION USED AS FLOOR FILL SHALL BE STYROFOAM HIGHLOAD 40 EXTRUDED POLYSTYRENE INSULATION (40 PSI COMPRESSIVE STRENGTH) ASTM C578, TYPE VI MANUFACTURED BY DOW CHEMICAL COMPANY, OR APPROVED EQUAL.
14. ALL EMBEDDED STEEL SHALL BE ASTM A36. ALUMINUM INSERTS ARE NOT PERMITTED.
15. CONSTRUCT EXPANSION JOINTS WHERE INDICATED. EXPANSION JOINT FILLER SHALL BE NONEXTRUDING BITUMINOUS TYPE PER ASTM D1751 INSTALL TO FULL DEPTH OF CONCRETE RECESSED TO ACCOMODATE JOINT SEALANT AND BACKER ROD WHERE NECESSARY.
16. CONCRETE COVERING OF REINFORCING STEEL (INCLUDING TIES AND STIRRUPS) SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS (SEE ACI 318 FOR CONDITIONS NOT NOTED):
    CONCRETE POURED AGAINST EARTH.................................. 3"
    CONCRETE EXPOSED TO EARH OR WEATHER:
    #5 OR SMALLER.. 1 1/2"
    #6 OR LARGER.. 2"
    CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
    COLUMNS (TIES AND MAIN REINFORCING).......................... 1 1/2"
    SLABS, WALLS, JOISTS:
    #14 OR #18 BARS.. 1 1/2"
    #11 OR SMALLER.. 1"
    BEAMS (STIRRUPS AND MAIN REINFORCING)..................... 1 1/2"
    ALL OTHER SURFACES NOT EXPOSED TO EARTH OR WEATHER.. 3/4"

RISE | FL architecture

ARCHITECT:

NYC OFFICE:
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE:
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHIMON GREENEBAUM, RA
SHIMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE:
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHIMON GREENEBAUM, RA
SHIMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
STRUCTURAL SPECIFICATIONS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:33:46 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted
DATE:
PROJECT NO. 2318_07

S002
DRAWING NO.

BARCODE

6/24/2024 8:16:00 AM

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

POST-INSTALLED ANCHORS

1. ALL POST INSTALLED ANCHORS SHALL BE HILTI UNLESS NOTED OTHERWISE ON PLAN.
2. ALL ALTERNATE FASTENER TYPE / MANUFACTURER SHALL BE SUBMIMTTED TO EOR FOR REVIEW / APPROVAL. SUBMITTALL SHALL INCLUDE DESIGN CALCULATIONS SIGNED & SEALED BY LICENSED PROFESSIONAL WITHIN PROJECT JURISDICTION.
3. POST-INSTALLED CONCRETE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.
4. POST INSTALLED ANCHORS SHALL BE INSTALLED IN MANNER THAT DOES NOT DAMAGE REINFORCING STEEL WITH CAST-IN-PLACE CONCRETE. CONTRACTOR SHALL SCAN ALL AREAS WITH REINFORCEMENT PRIOR TO INSTALLATION TO MITIGATE DAMAGE OF REINFORCEMENT.
5. NOTIFY EOR IF AS-BUILT LOCATION OF ANCHORAGE IS LARGER THEN 1/2" FROM LOCATION AS DESIGNATED WITHIN CONTRACT DOCUMENTS.
6. MECHANICAL ANCHORS (WEDGE / UNDERCUT) SHALL BE ANY OF THE FOLLOWING:
7. CHEMICAL ANCHORS (EPOXY SET) SHALL BE ANY OF THE FOLLOWING:
8. UNLESS NOTED OTHER, ALL DRILL & EPOXY SHALL REBAR SHALL USE HILTI HIT-HY-200 ADHESIVE, AND INSTALLED IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS. MINIMUM EMBEDMENT LENGTHS SHALL BE AS FOLLOWS:
9. ALL ANCHORS SHALL ASSUME CRACKED CONCRETE DESIGN CONDITION.
10. CONTRACTOR SHALL ARRANGE FOR HILTI REPRESENTATIVE TO PROVIDE ON SITE INSTALLATION TRAINING FOR EACH SPECIFIED ANCHOR TYPE, THE STRUCTURAL ENGINEERING OF RECORD SHALL RECEIVE DOCUMENATION VERIFY THAT ALL OF CONTRACTOR'S PERSONNEL INSTALLING ANCHORS HAVE BEEN TRAINED PRIOR TO COMMENCEMENT OF ANCHOR INSTALLATION.
11. CONCRETE SHALL HAVE ACHIEVED DESIGN STRENTH PRIOR TO INSTALLING POST-INSTALLED ANCHORS. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE THAT HAS CURED A MINIMUM OF 21 DAYS.
12. ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ANCHORS AND PROXIMITY OF ANCHORS TO EDGES OF CONCRETE / MASONRY. INSTALL ANCHOR IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON DRAWINGS.
13. POST-INSTALLED ANCHORS SHALL BE INSTALLED IN MANNER THAT DOES NOT DAMAGE REINFORCING STEEL, REINFORCING STEEL SHALL BE LOCATED BY NON-DESTRUCTIVE MEANS PRIOR TO DRILLING HOLES. WHERE THE ANCHOR LAYOUT CANNOT AVOID INTERFERENCE WITH REINFORCEMENT STEEL, THE CONTRACTOR SHALL NOTIFY STRUCTURAL ENGINEER TO OBTAIN AN ALTERNATIVE ANCHOR LAYOUT.
14. ADHESIVE ANCHOR SHALL BE INSTALLED WITH A 6" EMBEDMENT DEPTH UNLESS NOTED OTHERWISE. ANCHORS OTHER THEN ADHESIVE ANCHORS SHALL BE INSTALLED WITH EMBEDMENT DEPTH EQUAL TO MAXIMUM EMBEDMENT DEPTH NOTED IN THE MANUFACTURER'S PRODUCT TECHNICAL GUIDE UNLESS NOTED OTHERWISE ON PLAN. WHERE EMBEDMENT DEPTH IS SPECIFIED, THAT DEPTH IS REQUIRED FINAL EFECTIVE MINIMUM EMBEDMENT DEPTH.
15. POST INSTALLED ANCHORS SHALL BE INSPECTED PERIODICALLY DURING INSTALLATION.
16. POST INSTALLED ANCHORS IN VERTICAL AND OVERHEAD INSTALLATION ORIENTATIONS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION.
17. INSPECTIONS SHALL BE PERFORMED BY SPECIAL INSPECTOR WHO HAS BEEN APPROVED BY LOCAL BUILDING OFFICIAL. THE INSPECTOR SHALL VERIFY THAT ALL ANCHORS WERE INSTALLED IN ACCORDANCE WITH REQUIREMENTS OF THE CONTRACT DOCUMENTS, THE APPLICABLE ICC ESR REPORTS AND THE MANUFACTURER'S INSTALLATION MANUAL. INSPECTIONS SHALL INCLUDE VERIFICATION OF ANCHOR SPACING, EMBEDMENT AND EDGE DISTANCE REQUIREMENTS.

POST INSTALLED ANCHORS SCHEDULE

INSTALLATION TYPE	HILTI ANCHOR SELECTION
CONCRETE EXPANSION ANCHOR	KWIK BOLT TZ
CONCRETE ADHESIVE ANCHOR	HIT-HY 200 SAFE SET W/ HIT-Z ROD HIT-HY 200 W/ HOLLOW DRILL BIT W/ HAS-E ROD HIT-RE 500 SD W/ HAS-E ROD
CONCRETE SCREW ANCHOR	KWICK HUS EZ
CONCRETE DOWEL REINFORCEMENT	HIT-HY 200 SAFE SET W/ HOLLOW DRILL BIT HIT-RE 500 SD
CMU - GROUT FILLED EXPANSION ANCHOR	KWIK BOLT 3
CMU - GROUT FILLED SCREW ANCHOR	KWIK HUS EZ
CMU - GROUT FILLED ADHESIVE ANCHOR	HIT-HY 70 W/ HAS-E ROD
CMU - HOLLOW BLOCK ADHESIVE ANCHOR	HIT-HY 70 W/ HAS-E ROD & SCREEN TUBE

MASONRY NOTES

1. ALL MASONRY DESIGN & CONSTRUCTION, REINFORCED AND UNREINFORCED, SHALL COMPLY WITH THE REQUIREMENTS OF THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES", (TMS 402), AND THE "SPECIFICATION FOR MASONRY STRUCTURES," (TMS 602).
2. MASONRY UNITS SHALL BE MEDIUM WEIGHT HOLLOW CONCRETE UNITS CONFORMING TO THE REQUIREMENTS OF ASTM C90. UNITS SHALL NOT BE INSTALLED PRIOR TO ATTAINING THE REQUIRED 28 DAY STRENGTH.
3. MORTAR SHALL CONFORM TO ASTM C270, TYPE M OR S. ALL PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I. LIME SHALL CONFORM TO ASTM C207 AND MASONRY CEMENT SHALL CONFORM TO ASTM C91.
4. GROUT SHALL CONFORM TO ASTM C476 AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI. SLUMP OF GROUT SHALL BE 8 TO 10 INCHES AND THE MAXIMUM AGGREGATE SIZE SHALL BE 3/8" (AGGREGATE GRADED TO PRODUCE FINE GROUT IN CONFORMANCE WITH ASTM C476 AND C404).
5. HORIZONTAL JOINT REINFORCING: ASTM A82; 9-GAGE TRUSS-TYPE, GALVANIZED.
6. DEFORMED BAR REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60 AND SHALL BE FULL HEIGHT OF WALLS UNLESS OTHERWISE NOTED. PROVIDE BAR SPACERS AND POSITIONERS AS REQUIRED TO PROPERLY LOCATE AND STABILIZE REINFORCING DURING GROUTING OPERATIONS. GROUT ALL REINFORCED CELLS SOLID WITH GROUT.
7. SPLICES OF REINFORCING STEEL SHALL BE MADE ONLY AT THOSE LOCATIONS WHERE SPLICES ARE SHOWN ON THE STRUCTURAL DRAWINGS AND AT THOSE LOCATIONS WHERE SPLICES HAVE BEEN DETAILED ON THE REINFORCING STEEL PLACEMENT DRAWINGS THAT HAVE BEEN REVIEWED AND APPROVED BY THE ENGINEER OF RECORD.
8. PROVIDE VERTICAL MASONRY CONTROL JOINTS AT MAXIMUM 25'-0" ON CENTER UNLESS DETAILED ON ARCHITECTURAL DRAWINGS, COORDINATE LOCATIONS WITH ARCHITECT.
9. BOND BEAMS SHALL BE PROVIDED AT THE TOPS OF ALL CMU WALLS AND AT HORIZONTAL INTERVALS NOT TO EXCEED 10FT O.C. VERTICALLY. UNLESS INDICATED ON DRAWINGS.
10. VERTICAL CONTROL JOINTS SHALL BE PLACED SUCH THAT THE RATIO OF JOINT SPACING (L) DIVIDED BY WALL HEIGHT (H) DOES NOT EXCEED 3.0. IN NO CASE SHALL SPACING EXCEED 50 FT. CONTROL JOINTS SHALL BE CONSTRUCTED USING SASH BLOCKS AND DUR-O-WAL PREFORMED REGUUR RAPID CONTROL JOINT (OR EQUAL OF EXTRUDED RUBBER.) WALL REINFORCING SHALL BE DISCONTINUOUS AT JOINTS. VERTICAL JOINTS SHALL BE LOCATED AS FOLLOWS:
   A. CHANGES IN WALL HEIGHT OR THICKNESS.
   B. AT CONSTRUCTION/BUILDING EXPANSION JOINTS IN FOUNDATION, IN ROOF, AND IN FLOORS.
11. CONCRETE MASONRY UNITS SHALL BE LAID IN RUNNING BOND UNLESS INDICATED BY THE ARCHITECTURAL DRAWINGS. PROVIDE FULL BED AND HEAD JOINTS.
12. INSTALL FLASHING AT ALL CONDITIONS SUCH AS LINTELS AND SHELF ANGLES, WHERE THE DOWNWARD FLOW OF WATER WITHIN THE MASONRY WILL BE INTERRUPTED.
13. HOLLOW CONCRETE UNITS BELOW GRADE SHALL BE HAVE ALL CELLS GROUTED SOLID.
14. REINFORCE ALL BOND BEAMS WITH A MINIMUM 2 CONTINUOUS #5 BARS WITH MINIMUM 3000 PSI SMALL AGGREGATE CONCRETE (NOTE: MORTAR MIX DOES NOT CONSTITUTE GROUT). PROVIDE WALL ANCHORS TO ALL BUILDING COLUMNS AT MAXIMUM 48" VERTICAL AND AT ALL BOND BEAMS.
15. BOND BEAM UNITS SHALL BE OPEN CELL UNITS THAT PERMIT VERTICAL REINFORCING TO PASS THROUGH. WHERE BOND BEAMS COURSES STEP DUE TO SLOPING CONDITIONS, LAP REINFORCING A MINIMUM OF 4 FEET. PROVIDE MINIMUM BOND BEAM REINFORCING AS FOLLOWS, UNLESS NOTED OTHERWISE:
   A. EXTERIOR WALLS: (2) #4 x CONT. BELOW EACH FRAMING LEVEL.
   B. PARAPETS: (2) #4 x CONT. BELOW EACH FRAMING LEVEL.
   C. INTERIOR BEARING WALLS: (1) #5 x CONT. BELOW EACH FRAMING LEVEL.
   D. INTERIOR NON-LOAD BEARING WALLS: (2) #4 x CONT. BELOW EACH FRAMING LEVEL.
16. PROVIDE AND INSTALL TEMPORARY BRACING REQUIRED INSURING STABILITY OF ALL WALLS DURING CONSTRUCTION AND UNTIL ERECTION OF ATTACHED STRUCTURAL FRAMING IS COMPLETED.
17. PROVIDE GALVANIZED HORIZONTAL JOINT REINFORCEMENT IN ALL WALLS AND PARTITIONS AT 16" O.C. UNLESS OTHERWISE SHOWN OR NOTED. PROVIDE ONE (1) PIECE PREFABRICATED UNITS AT 8" O.C. AT ALL WALL CORNERS AND INTERSECTIONS.
18. ALL MORTAR JOINTS ON EXPOSED WALLS SHALL BE STRUCK TO PRODUCE A DENSE, SLIGHTLY CONCAVE SURFACE WELL BONDED TO THE SURFACE OF THE MASONRY UNIT.
19. ALLOW GROUT IN REINFORCED CMU WALLS TO CURE A MINIMUM OF 48 HOURS BEFORE IMPOSING CONCENTRATED OR OTHER LOADS FROM ABOVE.
20. PROVIDE MASONRY ANCHORS AT 16" O.C. SET ON COURSING AND ATTACHED TO ALL BEAMS, COLUMNS, PARTITIONS, AND WALLS ABUTTING OR EMBEDDED IN MASONRY UNLESS NOTED OTHERWISE ON ARCHITECTURAL AND STRUCTURAL DRAWINGS.
21. NO AIR-ENTRAINING ADMIXTURES OR ANTIFREEZE COMPOUNDS, SUCH AS CALCIUM CHLORIDE SHALL BE ADDED TO MORTAR.
22. ALL WALLS OR PILASTERS SUPPORTING STEEL AT BEARING PUTES SHALL BE GROUTED SOLID FOR FOUR COURSES IN DEPTH FOR A WIDTH OF 32".
23. DO NOT BACKFILL AGAINST FOUNDATION WALLS UNTIL MORTAR HAS ATTAINED MAXIMUM STRENGTH. WHERE BACKFILL IS PLACED AGAINST FOUNDATION WALLS BEFORE FLOOR CONSTRUCTION IS IN PLACE, PROVIDE TEMPORARY BRACING.
24. ALL MASONRY PIERS AND PARTITIONS SHALL BE TOOTHED TO ADJACENT MASONRY WALLS. PROVIDE TIES TO ADJACENT FLOOR AND ROOF CONSTRUCTION IN ACCORDANCE WITH DETAILS ON DRAWINGS.
25. THE CONTRACTOR SHALL VERIFY ALL OPENINGS BELOW LINTELS INDICATED ARE ADEQUATE TO ACCEPT DOORFRAMES, LOUVERS, ETC. AS SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS. NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES PRIOR TO LINTEL INSTALLATION.
26. PROVIDE ANCHORAGE TO ADJACENT STRUCTURAL STEEL FRAMING AT EACH FLOOR LEVEL AND ALONG EACH COLUMN. PROVIDE HOHMANN 4 BARNARD #359 AND VBT WALL TIES AT A MAXIMUM OF 24 INCHES ON CENTER HORIZONTALLY AND VERTICALLY.
27. CAVITIES CONTAINING REINFORCING OR BELOW BEARING PLATES SHALL BE GROUTED BY MEANS OF LOW-LIFT TECHNIQUES. HIGH-LIFT GROUTING MAY BE USED ONLY WITH PRIOR APPROVAL. FOLLOW ACI SPECIFICATIONS FOR MASONRY GROUTING.
28. ALL MASONRY WALLS SHALL BE ADEQUATELY BRACED DURING CONSTRUCTION TO RESIST WIND LOADS OF 25 PSF. NOTE THAT FLOOR AND ROOF DIAPHRAGMS WILL PROVIDE ULTIMATE STABILITY FOR WALLS. MASONRY WALLS SHALL NOT BE BUILT HIGHER THAN 10 TIMES THEIR THICKNESS WITHOUT BRACING.
29. ALL WALL SECTIONS AND PIERS LESS THAN TWO SQUARE FEET IN CROSS-SECTIONAL AREA SHALL BE FULLY GROUTED.
30. IMPLEMENT COLD WEATHER CONSTRUCTION PROCEDURES IN ACCORDANCE WITH TMS 402 WHEN AMBIENT TEMPERATURE FALLS BELOW 40 DEGREES F OR THE TEMPERATURE OF MASONRY UNITS IS BELOW 40 DEGREES F. WET OR FROZEN UNITS SHALL NOT BE LAID. THE TEMPERATURE OF THE NEWLY LAID MASONRY OR NEWLY GROUTED MASONRY SHALL BE MAINTAINED ABOVE 32 DEGREES F FOR A MINIMUM OF 24 HOURS USING THE METHODS DESCRIBED IN TMS 402.
31. IMPLEMENT HOT WEATHER CONSTRUCTION PROCEDURES IN ACCORDANCE WITH TMS 402 WHEN AMBIENT TEMPERATURE EXCEEDS 100°F, OR EXCEEDS 90°F WITH A WIND VELOCITY GREATER THAN 8 MPH.
32. GROUT PLACEMENT SHALL NOT START UNTIL THE PLACEMENT OF REINFORCING HAS BEEN APPROVED BY THE INSPECTION AGENCY.
33. SUBMIT PUBLISHED DATA FROM MANUFACTURERS OF PRODUCTS AND ACCESSORIES SPECIFIED, INDICATING COMPLIANCE WITH REQUIREMENTS.
34. PROVIDE MIX DESIGN AND TEST REPORTS FOR PRE-BLENDED MORTAR AND CONVENTIONAL GROUT INDICATING TYPES AND PROPORTIONS OF MATERIALS.
35. THE OWNER WILL ENGAGE A QUALIFIED INDEPENDENT TESTING AND INSPECTING AGENCY TO PERFORM FIELD TESTS AND INSPECTIONS AND PREPARE TEST REPORTS.
36. THE TESTING AND INSPECTION AGENCY SHALL MONITOR THE PROPORTIONING, MIXING, AND CONSISTENCY OF MORTAR AND GROUT; THE PLACEMENT OF MORTAR, GROUT, AND MASONRY UNITS; AND THE PLACEMENT OF REINFORCING STEEL FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS.

STRUCTURAL STEEL

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC 360 "STEEL CONSTRUCTION MANUAL" (LRFD), FIFTEENTH EDITION, 2016, INCLUDING SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, AND AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS.
2. STRUCTURAL STEEL SHAPES SHALL MEET THE FOLLOWING MINIMUM SPECIFICATIONS:
   A. FLANGE SHAPES..........ASTM A992 OR A572, GRADE 50
   B. STRUCTURAL SHAPES & PLATES..........ASTM A36, A572 OR A992
   C. STEEL PIPE..........ASTM A53, GRADE B
   D. STEEL TUBING (SQUARE OR RECT.).......... ASTM A500, GRADE C
   E. (ROUND)..........ASTM A501, GRADE B
3. GALVANIZED STRUCTURAL STEEL:
   A. STRUCTURAL SHAPES AND RODS..........ASTM A123.
   B. BOLTS, FASTENERS AND HARDWARE......ASTM A153.
4. RAISED PATTERN FLOOR PLATE: ASTM A786.
5. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36 UNLESS NOTED OTHERWISE.
6. STEEL MEMBERS SHOWN ON PLAN SHALL BE EQUALLY SPACED UNLESS NOTED OTHERWISE.
7. THE STEEL STRUCTURE IS A NON-SELF-SUPPORTING STEEL FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION OF THE METAL (ROOF/FLOOR) DECK AND ATTACHMENT TO THE MASONRY WALLS FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS REQUIRED FOR STABILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND ARE CAPABLE OF PROVIDING THIS SUPPORT.
8. CUTS, HOLES, COPING, ETC. REQUIRED FOR OTHER TRADES OR FIELD CONDITIONS SHALL BE SHOWN ON THE SHOP DRAWINGS AND MADE IN THE SHOP. CUTTING OR BURNING OF MAIN STRUCTURAL MEMBERS IN THE FIELD WILL NOT BE PERMITTED.
9. THE GENERAL CONTRACTOR AND STEEL ERECTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ANY FABRICATION OR ERECTION ERRORS OR DEVIATIONS AND RECEIVE WRITTEN APPROVAL BEFORE ANY FIELD CORRECTIONS ARE MADE.
10. BEAMS AND GIRDERS SHALL HAVE BEAM WEB HOLES AS INDICATED ON THE STRUCTURAL DRAWINGS. ALL HOLES SHALL BE CENTERED AT MID-DEPTH OF THE BEAM UNLESS OTHERWISE NOTED. ALL RECTANGULAR WEB HOLES SHALL HAVE A MINIMUM CORNER RADIUS OF 5/8" OR TWICE THE THICKNESS OF THE BEAM WEB, WHICHEVER IS GREATER. ALL WEB OPENINGS SHALL BE MACHINE OXYGEN CUT. MANUAL CUTTING OR BURNING IS NOT PERMITTED. COORDINATE LOCATION AND SIZE OF HOLE WITH MECHANICAL CONTRACTOR PRIOR TO REVIEW BY THE STRUCTURAL ENGINEER.
11. STEEL SHOP DRAWINGS SHALL BE COORDINATED WITH STAIR DETAILS. IF HANGER RODS ARE USED, PROVIDE FITTED WELDED STIFFENER PUTE 1/4" THICK MIN. ALONGSIDE HANGER LOCATION.
12. SPANDRELS AND COLUMNS ADJACENT TO MASONRY SHALL HAVE ADJUSTABLE MASONRY TIES.
13. SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED ON THE CONTRACT DOCUMENTS IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER AS TO LOCATION, TYPE OF SPLICE AND CONNECTION TO BE MADE.
14. BEAMS SHALL BE CAMBERED UPWARD WHERE SHOWN ON THE CONTRACT DOCUMENTS. WHERE NO UPWARD CAMBER IS INDICATED, ANY MILL CAMBER SHALL BE DETAILED UPWARD IN THE BEAMS. CAMBER INDICATED ON PLAN IS AFTER FINAL ERECTION INCLUDING MILL TOLERANCES.
15. HEADED CONCRETE ANCHORS SHALL BE NELSON HEADED CONCRETE ANCHORS (OR APPROVED EQUAL), AND SHALL CONFORM TO ASTM A108. ANCHORS SHALL BE AUTOMATICALLY END WELDED WITH SUITABLE STUD WELDING EQUIPMENT IN THE SHOP OR IN THE FIELD. WELDING SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE NELSON STUD WELDING COMPANY OR THE KSM WELDING SYSTEMS COMPANY. LENGTH OF STUDS SHALL BE HEIGHT OF METAL DECK + 1-1/2". STUD HEIGHT SHALL BE FINAL STUD HEIGHT AFTER MELT OFF.
16. DEFORMED BAR ANCHORS (D.B.A.) SHALL BE NELSON DEFORMED BAR ANCHORS (OR APPROVED EQUAL), AND SHALL BE MADE FROM COLD-DRAWN WIRE CONFORMING TO ASTM A1064. ANCHORS SHALL BE AUTOMATICALLY END WELDED WITH SUITABLE WELDING EQUIPMENT IN THE SHOP OR IN THE FIELD. WELDING SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE NELSON STUD WELDING COMPANY OR THE KSM WELDING SYSTEMS COMPANY.
17. PROVIDE WELDED STIFFENER PLATES ON BOTH SIDES OF THE WEB OF BEAMS AT POINTS OF CONCENTRATED LOADS INCLUDING BEAMS SUPPORTING COLUMNS OR RUNNING OVER THE TOPS OF COLUMNS, OR OTHER BEAMS. MINIMUM STIFFENER PLATE THICKNESS SHALL BE 3/8" OR FLANGE THICKNESS OF COLUMN ABOVE OR BELOW OR BEAM WEB THICKNESS ABOVE OR BELOW, WHICHEVER IS GREATER.
18. FIELD WELDED SURFACES WITHIN FOUR (4) INCHES OF WELD SHALL BE CLEANED AND GROUND SMOOTH. AFTER WELDING COAT THE EXPOSED AREA WITH APPROPRIATE PRIMER/PAINTS AS SPECIFIED.
   A. IF STEEL IS GALVANIZED, COAT THE EXPOSED AREA WITH GALVANIZING REPAIR PAINT. GALVANIZING REPAIR PAINT SHALL BE A HIGH ZINC DUST CONTENT PAINT COMPLYING WITH FEDERAL SPECIFICATIONS DOD-P-21035A OR SSPC-PAINT-20, COLD GALVANIZING COMPOUND BY ZRC PRODUCTS CO. OR EQUAL.
19. REFER TO ARCHITECTURAL DRAWINGS FOR FIREPROOFING REQUIREMENTS. DELETE PAINT ON ALL STEEL TO RECEIVE SPRAYED-ON FIREPROOFING OR CONCRETE ENCASEMENT.
20. ALL DISSIMILAR METALS SHALL BE TREATED OR PROPERLY SEPARATED TO PREVENT GALVANIC AND/OR CORROSIVE EFFECTS.
21. ALL STEEL SHALL BE PAINTED WITH SHOP STANDARD PRIMER UNLESS NOTED OTHERWISE.
22. STEEL ANGLES AND PLATES ALONG WITH BOLTS AND WASHERS, IN DIRECT CONTACT WITH EXTERIOR FINISH MASONRY, AND ALL EXTERIOR EXPOSED STRUCTURAL STEEL SHALL BE PAINTED WITH INORGANIC ZINC PRIMER
23. ALL EXPOSED STEEL (INCLUDING BUT NOT LIMITED TO DUNNAGE FRAMING, SCREEN WALL FRAMING, CANOPY FRAMING, ETC.) SHALL BE HOT DIP GALVANIZED. ANY POINTS OF WELDING SHALL BE TOUCHED UP IN THE FIELD WITH A ZINC-RICH PAINT BY THE STEEL ERECTOR.
24. ALL STEEL BELOW SLABS ON GRADE SHALL RECEIVE TWO (2) COATS OF BITUMINOUS PAINT - OR 3" MINIMUM CONCRETE COVER.
25. ALL DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED BY APPLYING AN ORGANIC ZINC REPAIR PAINT COMPLYING WITH THE REQUIREMENTS OF ASTM A780. GALVANIZING REPAIR PAINT SHALL CONTAIN 95% ZINC BY WEIGHT. THICKNESS OF APPLIED GALVANIZING REPAIR PAINT SHALL BE NO LESS THAN THE COATING THICKNESS REQUIRED BY ASTM A123 OR A153 AS APPLICABLE.
26. ALL STRUCTURAL STEEL INCLUDING BOLTS AND OTHER HARDWARE THAT IS SUBJECT TO WETTING WITH SALT-LADEN WATER OR OTHER MILD CHEMICAL ATTACK (SUCH AS INDOOR SWIMMING POOL AREAS) SHALL BE COMMERCIAL BLAST CLEANED AND PAINTED WITH THREE COATS OF EPOXY PAINT IN ACCORDANCE WITH STEEL STRUCTURES PAINTING COUNCIL PAINTING SYSTEM SPECIFICATION NO. 13.01. A URETHANE TOPCOAT SHALL BE PROVIDED FOR ALL STEEL EXPOSED TO VIEW.

STEEL CONNECTIONS (BOLTING / WELDING)

1. CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL MEET OR EXCEED THE REQUIREMENTS OF ASTM A325. BOLTS SHALL BE DESIGNED AS BEARING TYPE BOLTS, EXCEPT AS NOTED HEREIN OR ON PLAN. BEARING BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE "SNUG TIGHT" CONDITION AS OUTLINED IN THE AISC SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, AS COVERED IN ASTM F3125. BOLTS IN BRACING CONNECTIONS, MOMENT CONNECTIONS OR OTHER CONNECTIONS NOTED ON THE DRAWINGS ARE CONSIDERED TO BE SLIP-CRITICAL BOLTS AND SHALL BE TIGHTENED BY THE TURN-OF-NUT METHOD OR SHALL UTILIZE LOAD INDICATOR TYPE BOLTS, INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER. CONNECTION BOLTS SHALL HAVE HARDENED WASHER PLACED UNDER THE ELEMENT TO BE TIGHTENED.
2. THE FABRICATOR IS RESPONSIBLE FOR THE SELECTION, DESIGN AND DETAILING OF ALL CONNECTIONS NOT FULLY DETAILED IN THE CONTRACT DOCUMENTS. TYPICAL CONNECTION DETAILS ARE INDICATED ON THE DRAWINGS FOR DESIGN INTENT ONLY. THE FABRICATOR SHALL HAVE A REGISTERED PROFESSIONAL ENGINEER PREPARE THE CONNECTION DESIGNS, AND SUCH DESIGNS SHALL BE SUBMITTED FOR REVIEW WITH THE SHOP DRAWINGS. CONNECTIONS SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH THE AISC "MANUAL OF STEEL CONSTRUCTION". THE END REACTION OF THE CONNECTIONED BEAM SHALL BE DETERMINED AS SPECIFIED IN NOTE 3. ALL BEAM TO COLUMN CONNECTIONS SHALL BE DESIGNED FOR THE MINIMUM SHEAR REACTION INDICATED ABOVE, IN ADDITION TO A NON-CONCURRENT AXIAL FORCE THAT IS THE GREATER OF: 10 KIPS, 2/3 SHEAR REACTION, OR 2% OF COLUMN LOAD (ACTING IN BOTH TENSION AND COMPRESSION).
3. STEEL CONNECTIONS IN HIGH-RISE CONSTRUCTION AS DEFINED BY THE INTERNATIONAL BUILDING CODE SHALL BE DESIGN FOR THE MINIMUM SHEAR REACTIONS INDICATED IN NOTE 2, IN ADDITION TO A NON-CONCURRENT AXIAL FORCE THAT IS THE GREATER OF:
   - 10 KIPS
   - 2% OF COLUMN LOAD (ACTING IN BOTH TENSION AND COMPRESSION)
   - 2/3 SHEAR REACTION
4. BEAM SHEAR CONNECTIONS SHALL BE DESIGNED TO SUPPORT THE LRFD (FACTORED) LOAD LEVEL SHEAR REACTIONS INDICATED ON DRAWINGS. DETAILER / FABRICATOR SHALL CONTACT E.O.R. FOR ANY BEAM REACTIONS NOT SPECIFICALLY DESIGNATED ON PLAN. BEAM WEB SHEAR CONNECTIONS SHALL BE DETAILED SO THAT THE LENGTHS OF THE CONNECTION PLATES OR ANGLES ARE NO LESS THAN ONE-HALF OF THE "T" DIMENSION OF THE SUPPORTED BEAMS. WHERE REACTIONS ARE NOT SHOWN, CONNECTIONS SHALL BE DETAILED TO SUPPORT THE FACTORED-LOAD LEVEL REACTIONS INDICATED IN THE TABLE BELOW:

BEAM SIZE	R_U
W8, W10, W12	30k
W14	35k
W16	45k
W18	65k
W21	85k
W24	95k

BEAM SIZE	R_U
W27	105k
W30	115k
W33	125k
W36	150k
W40	165k

5. CONNECTION DESIGN RESPONSIBILITY SHALL INCLUDE CALCULATION OF MEMBER STRENGTH AT CONNECTIONS CONSIDERING THE EFFECTS OF COPES, BOLT HOLES CONNECTION ECCENTRICITY AND CONNECTION GEOMETRY AND SHALL CONSIDER ALL LIMIT STATES INCLUDING BUT NOT LIMITED TO REVIEW OF GROSS SHEAR,NET SHEAR, BLOCK SHEAR, WEB TEAR-OUT, BEARING, FLEXURAL STRENGTH, LOCAL BUCKLING,TENSILE STRENGTH THROUGH BOTH THE NET AND GROSS SECTIONS, COMPRESSIVE STRENGTH AND DUCTILITY. DESIGN OF CONNECTIONS SHALL BE IN ACCORDANCE WITH RECOGNIZED PUBLISHED METHODS SUCH AS THOSE PUBLISHED IN THE AISC "ENGINEERING JOURNAL", THE AISC STEEL CONSTRUCTION MANUAL AND THE AISC STEEL CONSTRUCTION MANUAL DESIGN EXAMPLES. CONNECTION DESIGN SHALL CONSIDER TRANSFER FORCES THROUGH CONNECTED AND CONNECTING MEMBERS. CONNECTION DESIGN RESPONSIBILITY SHALL INCLUDE ANALYSIS AND DESIGN OF PLATES, BRACKETS, STRUTS, STIFFENER PLATES, GUSSET PLATES AND OTHER ELEMENTS TO TRANSFER FORCES INTO AND BETWEEN MEMBERS. MANUAL CALCULATIONS SHALL BE SUBMITTED FOR EACH CONNECTION TYPE TO VERIFY THAT CONNECTIONS DESIGNED USING COMPUTER SOFTWARE CONSIDER ALL LIMIT STATES AND PRODUCE RESULTS IDENTICAL TO THE MANUAL CALCULATIONS.
6. WELDERS SHALL HAVE CURRENT EVIDENCE OF PASSING THE APPROPRIATE AWS QUALIFICATION TESTS. THE ENGINEER MAY REQUEST SUCH EVIDENCE AT ANY TIME DURING THE PROJECT.
7. WELDING SHALL CONFORM TO THE LATEST AMERICAN WELDING SOCIETY STANDARD D1.1. I-80 ELECTRODES FOR SHOP AND FIELD WELDS SHALL CONFORM TO AWS A5.1 OR AWS A5.5 CLASS E70XX, LOW HYDROGEN. MINIMUM WELD SIZE SHALL BE 3/16" UNLESS NOTED OTHERWISE.
8. WELDING TO THE EXISTING STEEL WILL NOT BE ALLOWED AND THE CONTRACTOR SHALL ANTICIPATE USING FIELD BOLTED CONNECTIONS TO THE EXISTING STEEL.
9. ALL BRACING OR TRUSS CONNECTIONS, WHICH HAVE NOT BEEN SPECIFICALLY DETAILED, SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION FOR THE FORCES NOTED ON THE ELEVATIONS AND DETAILS. THIS SHALL INCLUDE ALL GUSSET PLATES, FILLER PLATES, ANGLES, STIFFENERS, BOLTS OR WELDS, OR OTHER MATERIAL REQUIRED FOR THE CONNECTION. STAMPED CALCULATIONS FOR THE CONNECTION DESIGN SHALL BE SUBMITTED ALONG WITH THE SHOP DRAWINGS FOR REVIEW BY THE ENGINEER.
10. GUSSET PLATE CONNECTIONS SHALL BE SIZED FOR 100% OF THE AXIAL FORCES INDICATED ON THE ELEVATIONS. DESIGN ALL GUSSET PLATES AND CONNECTORS AS REQUIRED FOR COMPLIANCE WITH AISC. PROVIDE STIFFENER PLATES AS REQUIRED AT THE GUSSET PLATE CONNECTIONS.
11. THE NET AREA (REFER TO AISC SECTION B2 AND B3) AT THE CONNECTION OF ANY BRACING MEMBER SHALL NOT BE LESS THAN 85 PERCENT OF THE GROSS CROSS SECTIONAL AREA OF THE MEMBER. ADDITIONAL PLATES SHALL BE ADDED AS NECESSARY TO MAINTAIN THE MINIMUM NET CROSS SECTIONAL AREA. SUCH PLATES SHALL EXTEND A MINIMUM DISTANCE EQUAL TO THE DEPTH OF THE MEMBER PAST THE LAST ROW OF BOLTS.
12. ALL CONNECTIONS SHALL BE SYMMETRICAL ABOUT THE AXIS OF THE MEMBER CONNECTED. PROVIDE ONLY ONE GRADE OF BOLT FOR EACH BOLT DIAMETER TO BE USED IN THE CONNECTIONS. DO NOT MIX GRADE OF BOLTS.
13. PRIOR TO DETAILING CONNECTIONS FOR STRUCTURAL STEEL, THE STEEL FABRICATOR SHALL SUBMIT FOR REVIEW REPRESENTATIVE DETAILS AND CALCULATIONS FOR EACH TYPE OF STRUCTURAL STEEL CONNECTION TO BE UTILIZED. AFTER REVIEW, THE CONNECTIONS MAY BE INCORPORATED INTO SHOP DRAWINGS, ALONG WITH A TABLE OF DESIGN CAPACITIES FOR THE RANGE OF CONNECTIONS TO BE USED.
14. VISUALLY INSPECT ALL FILLET WELDS. 10% OF ALL FIELD FILLET WELDS IN PRIMARY CONNECTIONS AND MULTI-PASS WELDS SHALL BE TESTED BY THE MAGNETIC PARTICLE METHOD, COMPLYING WITH ASTM E709, PERFORMED ON THE ROOT PASS AND ON THE FINISHED WELD.
15. 100% OF FULL PENETRATION WELDS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164.
16. 100% OF WELDS IN BEAM AND COLUMN MOMENT CONNECTIONS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164.
17. REPORTS OF EACH TEST SHALL BE GIVEN TO THE STRUCTURAL ENGINEER. NO FAILED WELD SHALL BE PERMITTED TO REMAIN IN SERVICE. IT IS THE RESPONSIBILITY OF THE TESTING UBORATORY TO PROVIDE TIMELY NOTICE OF FAILED TESTS TO THE CONTRACTOR.

STEEL BAR JOISTS

1. DESIGN OF STEEL BAR JOISTS SHALL BE IN ACCORDANCE LATEST STEEL JOIST INSTITUTE (SJI) SPECIFICATIONS. DESIGN OF JOISTS SHALL BE COMPLETED BY LICENSED ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION IN CONJUNCTION WITH THE DESIGNATED LOADING CONDITIONS NOTED WITHIN THESE DOCUMENTS. ALL SUBMISSIONS SHALL BEAR THIS ENGINEER'S SEAL AND SIGNATURE.
2. OPEN WEB JOISTS SHALL CONFORM TO THE "STANDARD SPECIFICATION AND LOAD TABLES FOR OPEN WEB STEEL JOISTS" OF THE STEEL JOIST INSTITUTE.
3. SPECIAL JOISTS (SP, KCS), WHERE INDICATED ON PLANS HAVE SPECIAL DESIGN REQUIREMENTS. REFERENCE FRAMING PLANS FOR LOCATION OF SPECIAL JOISTS AND APPROPRIATE JOIST DIAGRAMS FOR DESIGN LOADS AND OTHER SPECIFICATIONS.
4. JOISTS ALONG COLUMN LINES SHALL BE INSTALLED WITH ERECTION BOLTS AND WELD AFTER COMPLETION OF PLUMBING AND ALIGNMENT. ALL OTHER BAR JOISTS TO STRUCTURAL STEEL CONNECTIONS SHALL BE WELDED. PROVIDE BOTTOM CHORD EXTENSIONS OF JOISTS FRAMING ALONG COLUMNS AND TO BRACE TRUSS GIRDERS AS REQUIRED BY OSHA.
5. JOIST BRIDGING SHALL BE INSTALLED IN ACCORDANCE WITH LATEST STEEL JOISTS INSTITUTE SPECIFICATION. BRIDGING SHOWN WITHIN DRAWINGS SHALL BE CONSIDERED AS THE MINIMUM REQUIREMENTS. JOIST MANUFACTURER COMPLY WITH OSHA REQUIREMENTS AND SUPPLY ADDITIONAL BRIDGING AS REQUIRED.
6. BRIDGING SHALL BE WELDED OR BOLTED AND ANCHORED AT END WALLS OR BEAMS. ALL BRIDGING, BRIDGING ANCHORS, AND JOIST CONNECTIONS SHALL BE COMPLETELY INSTALLED PRIOR TO THE APPLICATION OF ANY CONSTRUCTION LOADS.
7. ROOF JOISTS SHALL BE DESIGNED TO RESIST NET WIND UPLIFT LOADS. PROVIDE BRIDGING AT THE FIRST PANEL POINT FROM SUPPORTS AND ADDITIONAL BRIDGING AS REQUIRED BY THE BAR JOIST ENGINEER'S DESIGN.
8. LOADS EXCEEDING 100 LBS SHALL NOT BE HUNG FROM THE JOISTS UNLESS THE LOAD IS APPLIED WITHIN 6 INCHES FROM A PANEL POINT OR THE JOIST IS PROPERLY STRENGTHENED. REINFORCEMENT OF STEEL BAR JOIST FOR LOCATIONS GREATER THEN 6" AWAY FROM PANEL POINTS SHALL BE COMPLETED IN ACCORDANCE WITH SJI STANDARDS.
9. OPEN WEB STEEL JOISTS AND BRIDGING FOR JOISTS SHALL CONFORM, TO THE STEEL JOIST INSTITUTE "STANDARD SPECIFICATION FOR OPEN WEB JOISTS". TOP CHORDS SHALL BE ANGLES OR TEES. BRIDGING SHALL BE CONTINUOUS THROUGH OPEN WEB STEEL JOISTS AND BE ANCHORED TO SPANDREL MEMBERS. JOISTS SHALL BE WELDED TO SUPPORTING STEEL MEMBERS. WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STEEL BEAMS, THE JOISTS AT THE COLUMN LINE SHALL BE BOLTED TO SUPPORTING MEMBERS DURING ERECTION.
10. THE OPEN WEB STEEL JOIST MANUFACTURER SHALL BE AN SJI APPROVED FABRICATOR. AS MAY BE REQUIRED BY THE BUILDING OFFICIAL, TESTING AND INSPECTION OF THE OPEN WEB JOISTS SHALL BE PERFORMED BY AN APPROVED AGENCY.

METAL DECK

1. GAUGE METAL DECKING SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH "DESIGN MANUAL FOR FLOOR DECKS AND ROOF DECKS", STEEL DECK INSTITUTE (SDI). ALL COMPOSITE STEEL FLOOR DECK SHALL BE IN CONFORMANCE WITH THE "SPECIFICATIONS FOR COMPOSITE STEEL FLOOR DECK" OF THE STEEL DECK INSTITUTE, LATEST EDITION.
2. DECK PROPERTIES ARE BASED ON PRODUCTS MANUFACTURED BY VULCRAFT STEEL ROOF & FLOOR DECK, ISSUED 2008. DECKS BY OTHER MANUFACTURER'S MAY BE SUPPLIED PROVIDED LOAD CARRYING CAPACITY BASED ON MANUFACTURER'S STANDARD LOAD TABLES, DEFLECTION CHARACTERISTICS, AND UL FIRE RATINGS EQUAL OR EXCEED THOSE OF MATERIALS SPECIFIED AND IF APPROVED BY THE ARCHITECT AND STRUCTURAL ENGINEER.
3. COMPOSITE, NON-COMPOSITE AND ROOF DECKING SHALL CONFORMING TO ASTM A 611 GRADE C AND D OR A 653 OR HIGHER SPECIFICATIONS WITH A MINIMUM YIELD STRENGTH OF 33 KSI.
4. INSTALL METAL DECK IN ACCORDANCE WITH SDI SUGGESTED SPECIFICATIONS UNLESS NOTED OTHERWISE ON THE DRAWINGS. INDIVIDUAL DECK SHEETS SHALL EXTEND OVER AT LEAST (3) SPANS, WITH STANDARD DECKS LAPS TO BE PLACED OVER SUPPORTS.
5. WHERE PARTIAL PANELS MAY BE REQUIRED TO COMPLETE DECK INSTALLATION AT PERIMETER OF STRUCTURE, PROVIDE WELDS IN EACH FLUTE TO STRUCTURAL MEMBERS. INSTALL DECK IN THREE CONTINUOUS SPAN LENGTHS.
6. NON-COMPOSITE AND ROOF DECKING SHALL BE WELDED TO STEEL SUPPORTS, INCLUDING THE EDGE SUPPORT PARALLEL TO THE DECK SPAN WITH 5/8" DIAMETER (EFFECTIVE FUSION DIAMETER) PLUG WELDS. FASTEN SIDE LAPS WITH #10 SELF-TAPPING SCREWS.
7. DECK SUPPLIER SHALL PROVIDE ALL ADDITIONAL FRAMING, CLOSURE ANGLES AND PLATES, POUR STOPS, SCREED ANGLES, AND ROOF SUMP PANS AS REQUIRED AT THE EDGES OF ALL OPENINGS AND AT ALL SLAB DEPRESSIONS, OR CHANGES OF DECK DIRECTION, INCLUDING THOSE WHICH HAVE NOT BEEN DETAILED.
8. ATTACH SHEETS TO STEEL SUPPORT MEMBERS AS INDICATED AND IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS FOR INSTALLATION. WHEN DECK IS SCHEDULED TO BE EXPOSED, DE-SLAG, CLEAN AND TOUCH UP WELDS WITH A ZINC-RICH PRIMER.
9. LAP ROOF AND FLOOR DECK ENDS MINIMUM OF 2 INCHES. WHEN FASTENING DECK TO SUPPORT MEMBERS PROVIDE WELDING MATERIALS INSTALLATION PROCEDURES TO PREVENT BURNING OF HOLES IN DECK.
10. PROVIDE SIX INCH CLOSURE STRIP WHERE CHANGES IN DECK DIRECTION OCCUR. CLOSURE TO BE SAME GAGE AS DECK.
11. AT PERIMETER OF DECK, SECURE DECK TO STRUCTURAL MEMBERS WITH SAME ATTACHMENT AND SPACING SUPPORT ATTACHMENT AS INDICATED ON PLANS.
12. ALL STEEL FLOOR DECK SHALL BE WELDED TO ALL SUPPORTING STEEL ELEMENTS. WELDING WASHERS SHALL BE USED AS REQUIRED BY THE DECK MANUFACTURER.
13. STEEL DECK SUPPLIER SHALL SUBMIT SHOP DRAWINGS INDICATING THE SHEAR STUD PLACEMENT.
14. PRIOR TO AND DURING CONCRETE PLACEMENT, THE FLOOR DECK SHALL BE PLANKED TO PREVENT DAMAGE TO THE DECK. CONCENTRATED AND IMPACT LOADS SHALL BE AVOIDED.
15. SHEAR CONNECTORS SHALL BE HEADED STUDS CONFORMING TO ASTM A108, GRADES 1010, 1015, 1017, OR 1020. SHEAR CONNECTORS SHALL BE MACHINE WELDED TO STEEL.
16. SHEAR CONNECTORS SHALL BE EQUALLY SPACED OVER THE LENGTH OF THE BEAM UNLESS NOTED OTHERWISE. WHERE THE NUMBER OF STEEL DECK CORRUGATIONS AVAILABLE IS LESS THAN THE NUMBER OF SHEAR CONNECTORS REQUIRED, USE PAIRS OF SHEAR CONNECTORS STARTING FROM EACH END OF THE BEAM AND CONTINUING TOWARD THE CENTER UNTIL IT IS POSSIBLE TO RETURN TO A SINGLE SHEAR CONNECTOR IN EACH CORRUGATION.
17. NO MECHANICAL OR ELECTRICAL PIPING, FIXTURES, UNITS OR SYSTEMS MAY BE HUNG DIRECTLY FROM THE ROOF DECK.

RISE | FL architecture

ARCHITECT:

NYC OFFICE:
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE:
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHIMON GREENEBAUM, RA
SHIMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE:
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHIMON GREENEBAUM, RA
SHIMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:
JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
STRUCTURAL SPECIFICATIONS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:37:38 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted
DATE:
PROJECT NO. 2318_07
S003
DRAWING NO.

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:00 AM

TENSION DEVELOPMENT LENGTHS (LD) (CLASS A) (IN)

BAR #	f'c							
	3000	4000	5000	6000	7000	8000	9000	10000+
#3	17	15	13	12	12	12	12	12
#4	22	19	17	16	15	14	13	12
#5	28	24	22	20	18	17	16	15
#6	33	29	26	24	22	20	19	18
#7	48	42	37	34	32	30	28	27
#8	55	48	43	39	36	34	32	30
#9	62	54	48	44	41	38	36	34
#10	70	61	54	49	46	43	40	38
#11	78	67	60	55	51	48	45	43

TOP BAR TENSION DEVELOPMENT LENGTHS (LD) (CLASS A) (IN)

BAR #	f'c							
	3000	4000	5000	6000	7000	8000	9000	10000+
#3	22	19	17	15	14	13	13	12
#4	29	25	22	20	19	18	17	16
#5	36	31	28	25	24	22	21	20
#6	43	37	33	31	28	26	25	24
#7	63	54	48	44	41	38	36	34
#8	71	62	55	51	47	44	41	38
#9	81	70	63	57	53	48	47	44
#10	91	78	70	64	59	56	53	50
#11	101	87	78	71	66	62	58	55

TENSION DEVELOPMENT LENGTHS (1.3LD) (CLASS B) (IN)

BAR #	f'c							
	3000	4000	5000	6000	7000	8000	9000	10000+
#3	22	19	17	15	14	13	13	12
#4	29	25	22	20	19	18	17	16
#5	36	31	28	25	24	22	21	20
#6	43	37	33	31	28	26	25	24
#7	63	54	48	44	41	38	36	34
#8	71	62	55	51	47	44	41	38
#9	81	70	63	57	53	48	47	44
#10	91	78	70	64	59	56	53	50
#11	101	87	78	71	66	62	58	55

TOP BAR TENSION DEVELOPMENT LENGTHS (1.3LD) (CLASS B) (IN)

BAR #	f'c							
	3000	4000	5000	6000	7000	8000	9000	10000+
#3	28	24	22	20	18	17	16	16
#4	37	32	28	26	25	23	22	21
#5	47	40	36	33	31	29	27	26
#6	56	48	43	40	37	34	32	31
#7	81	70	63	58	53	50	47	45
#8	93	80	72	66	61	57	54	51
#9	105	91	81	74	68	64	61	57
#10	118	102	91	83	77	72	68	65
#11	131	113	101	93	86	80	76	72

NOTES:
1. TENSION DEVELOPMENT LENGTH SCHEDULES CONFORM TO ACI 318. TABULATED VALUES ASSUME GRADE 60 UNCOATED REINFORCEMENT AND NORMAL WEIGHT CONCRETE.
2. "TOP" BAR SPLICE LENGTHS IS DEFINED BY ACI 318 AS HAVING MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BAR. TOP BARS INCLUDE HORIZONTAL BARS OF BEAMS, SLABS, WALLS, FOOTINGS, ECT
3. LENGTHS TABULATED MUST BE MULTIPLIED BY THE FOLLOWING MODIFICATION FACTORS
   A. LOW COVER OR SMALL SPACING: IF EITHER CRITERIA BELOW ARE MET.....1.5
      1) CONCRETE COVER LESS THAN BAR DIA
      2) CENTER TO CENTER SPACING LESS THAN 2x BAR DIA
   B. LIGHT WEIGHT CONCRETE 1.33
   C. EPOXY - COATED BARS:
      1) BARS WITH COVER < 3db, OR WITH CLEAR SPACING < 6db ..... 1.5 FOR BOTTOM & VERTICAL BARS. 1.3 FOR "TOP" BARS*
      2) ALL OTHER CONDITIONS 1.2
      * FOR EPOXY-COATED "TOP" BARS, THE MAXIMUM COMBINED FACTORS = 1.7
4. LAP SPLICES ARE NOTE PERMITTED FOR #14 AND #18 BARS. USE MECHANICAL CONNECTORS
5. CONDITIONS WHICH REQUIRE CASE 2 SPLICE LENGTHS SHOULD BE AVOIDED IF AT ALL POSSIBLE FOR #7 BARS OR LARGER. THESE LONG LENGTHS PRESENT CONSTRUCTIBILITY PROBLEMS DUE TO CONGESTION.

COMPRESSION LAP SPLICES (IN)

BAR #	60 KSI (30db)	75 KSI (44db)	80 KSI (48db)
#3	12	17	18
#4	15	22	24
#5	19	28	30
#6	23	33	36
#7	27	38	42
#8	30	44	48
#9	34	50	55
#10	38	56	61
#11	43	62	68

NOTES:
1. TABULATED VALUES ASSUME NORMAL WEIGHT CONCRETE WITH MINIMUM f'c OF 3,000 PSI AND UNCOATED REINFORCEMENT.
2. LAP SPLICES ARE NOT PERMITTED FOR # 14 AND #18. USE MECHANICAL CONNECTIONS OR WELDED SPLICES.
3. LAP SPLICES OF #14 AND #18 BAST TO # 11 AND SMALLER BARS ARE PERMITTED PER ACI 318.

COMPRESSION DEVELOPMENT LENGTHS (NORAMAL WEIGHT CONCRETE) (IN)

BAR #	Fy = 60,000psi			Fy = 75,000psi			Fy = 80,000 psi		
	f'c			f'c			f'c		
	3000	4000	5000+	3000	4000	5000+	3000	4000	5000+
#3	9	7	7	11	9	9	11	10	9
#4	11	10	9	14	12	12	15	13	12
#5	14	12	12	17	15	14	19	16	15
#6	17	15	14	21	18	17	22	19	18
#7	19	17	16	24	21	20	26	22	21
#8	22	19	18	28	24	23	30	26	24
#9	25	22	21	31	27	26	33	29	27
#10	28	24	23	35	30	29	37	32	31
#11	31	27	26	39	34	32	41	36	34
#14	37	32	31	47	40	38	50	43	41

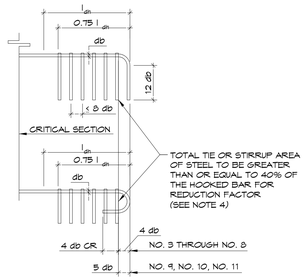
COMPRESSION DEVELOPMENT LENGTHS (LIGHT WEIGHT CONCRETE) (IN)

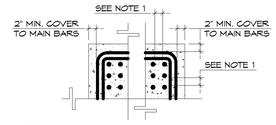
BAR #	Fy = 60,000psi						Fy = 75,000psi						Fy = 80,000 psi					
	f'c						f'c						f'c					
	3000	4000	5000	6000	7000	8000+	3000	4000	5000	6000	7000	8000+	3000	4000	5000	6000	7000	8000+
#3	11	10	9	8	7	7	14	12	11	10	9	9	15	13	12	11	10	9
#4	15	13	12	11	10	9	19	16	14	13	12	12	20	17	15	14	13	12
#5	19	16	14	13	12	12	23	20	18	16	14	14	25	21	19	18	16	15
#6	22	19	17	16	15	14	28	24	22	20	17	17	30	26	23	21	19	18
#7	26	22	20	18	17	16	32	28	25	23	20	20	34	30	27	24	23	21
#8	30	26	23	21	19	18	37	32	29	26	23	23	39	34	30	28	26	24
#9	33	29	26	24	22	21	41	36	32	29	26	26	44	38	34	31	29	27
#10	37	32	29	27	25	23	47	40	36	33	29	29	50	43	39	35	33	31
#11	41	36	32	29	27	26	52	45	40	37	32	32	55	48	43	39	36	34
#14	50	43	39	35	33	31	62	54	48	44	38	38	66	57	51	47	43	41
#18	66	57	51	47	43	41	83	72	64	58	51	51	88	76	68	62	58	54

STANDARD HOOKED BAR TENSION DEVELOPMENT LENGTH (IN)

BAR #	f'c							
	3000	4000	5000	6000	7000	8000	9000	10000+
#3	8	7	7	7	6	6	6	6
#4	12	11	10	10	10	9	8	8
#5	16	15	15	14	13	12	12	11
#6	21	20	19	19	17	16	15	14
#7	26	25	24	23	22	20	19	18
#8	32	30	29	28	26	25	23	22
#9	38	36	35	34	32	30	28	26
#10	46	43	42	41	38	35	33	32
#11	54	50	48	47	44	41	39	37
#14	70	66	64	62	58	54	51	48
#18	108	102	98	96	89	83	78	74

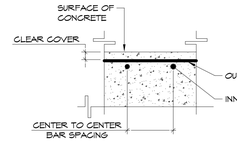
NOTES:
1. TABULATED VALUES ASSUME GRADE 60 UNCOATED REINFORCEMENT AND NORMAL WEIGHT CONCRETE.
2. LENGTHS TABULATED MUST BE MULTIPLIED BY THE FOLLOWING MODIFICATION FACTORS:
   A. LIGHT WEIGHT CONCRETE....1.33
   B. EPOXY COATED BARS.....1.2
3. FOR #11 BARS AND SMALLER, IF SPACING OF HOOKED BARS IS GREATER THAN 6db, OR IF THE STIRRUP REQUIREMENTS SHOWN IN DETAIL ARE MET THEN VALUES MAY BE REDUCED BY 0.625.



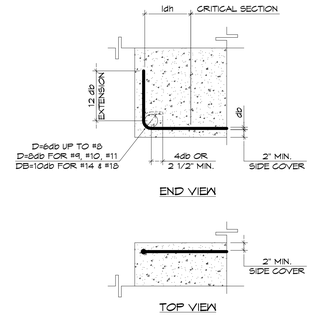


NOTES:
1. MIN. CLEAR BETWEEN REBAR LAYERS IS THE:
   - GREATER OF 1" AND db, FOR TYP MEMBERS;
   - GREATER OF 1.5" AND 1.5db FOR COMPRESSION MEMBERS.

MULTIPLE REBAR LAYERS



TYPICAL REBAR CALLOUTS



STANDARD END HOOK

TYPICAL CONCRETE COVER FOR REINFORCING BARS

FOUNDATION ELEMENTS	
CONCRETE POURED AGAINST EARTH	3"
CONCRETE EXPOSED TO EARTH OR WEATHER (#5 OR SMALLER)	1-1/2"
CONCRETE EXPOSED TO EARTH OR WEATHER (#6 OR LARGER)	2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
COLUMNS (TIES)	1-1/2"
SLABS, WALLS, AND JOISTS (#14 OR #18 BARS)	1-1/2"
SLABS, WALLS, AND JOISTS (#11 OR SMALLER)	1"
BEAMS (STIRRUPS)	1-1/2"
ALL OTHER SURFACES NOT EXPOSED TO EARTH OR WEATHER	3/4"

MASONRY BLOCK REQUIREMENTS

MASONRY UNIT COMPRESSIVE STRENGTH	NET AREAS COMPRESSIVE STRENGTH (f'm)
2,000 psi	2,000 psi
2,600 psi	2,250 psi
3,250 psi	2,500 psi
3,900 psi	2,750 psi
4,500 psi	3,000 psi

NOTES:
1. NET COMPRESSIVE STRENGTH ARE BASED ON M OR S TYPE GROUT

MASONRY LAP SPLICE SCHEDULE (f'm = 2,000 psi)

MASONRY LAP SPLICE SCHEDULE (f'm = 2,000 psi)

BAR SIZE	6" WALL	8" WALL	10" WALL	12" WALL
#3	12"	12"	12"	12"
#4	18"	13"	12"	12"
#5	NS	20"	16"	13"
#6	NS	30"	24"	24"
#7	NP	52"	40"	33"
#8	NP	12"	61"	50"
#9	NP	NP	78"	64"

MASONRY LAP SPLICE SCHEDULE (f'm = 2,500 psi)

MASONRY LAP SPLICE SCHEDULE (f'm = 2,500 psi)

BAR SIZE	6" WALL	8" WALL	10" WALL	12" WALL
#3	12"	12"	12"	12"
#4	16"	12"	12"	12"
#5	NS	18"	14"	12"
#6	NS	34"	28"	21"
#7	NP	47"	36"	29"
#8	NP	71"	55"	45"
#9	NP	NP	71"	57"

MASONRY LAP SPLICE SCHEDULE (f'm = 2,750 psi)

MASONRY LAP SPLICE SCHEDULE (f'm = 2,750 psi)

BAR SIZE	6" WALL	8" WALL	10" WALL	12" WALL
#3	12"	12"	12"	12"
#4	15"	12"	12"	12"
#5	NS	17"	13"	12"
#6	NS	32"	25"	21"
#7	NP	44"	34"	28"
#8	NP	60"	52"	42"
#9	NP	NP	67"	55"

MASONRY LAP SPLICE SCHEDULE (f'm = 3,000 psi)

MASONRY LAP SPLICE SCHEDULE (f'm = 3,000 psi)

BAR SIZE	6" WALL	8" WALL	10" WALL	12" WALL
#3	12"	12"	12"	12"
#4	14"	12"	12"	12"
#5	NS	16"	13"	12"
#6	NS	31"	24"	20"
#7	NP	42"	33"	27"
#8	NP	65"	50"	41"
#9	NP	NP	64"	52"

NOTES:
1. LAPS ARE APPLICABLE TO ONLY REBAR CENTERED IN THE WIDTH OF CMU WALL.
2. TABLE IS BASED ON Fy = 60,000 PSI FOR GRADE 60 STEEL. APPLIES TO BOTH ALLOWABLE STRESS DESIGN AND STRENGTH DESIGN.
3. NP INDICATES REBAR NOT PERMITTED FOR USE IN CMU WALL WIDTH.
4. NS INDICATES REBAR THAT CANNOT BE SPLICED IN CMU WALL WIDTH.
5. BARS LARGER THAN NO. 9 TO BE SPLICED BY MECHANICAL CONNECTORS.
6. LENGTHS TABULATED MUST BE MULTIPLIED BY 1.5 FOR EPOXY COATED REBAR. DEVELOPMENT LENGTH NOT TO EXCEED 12"Db.

MASONRY SCHEDULES

CONCRETE SCHEDULES

RISE | FL architecture

ARCHITECT

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEFLARCHITECTURE.COM
516.249.3746

NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENEBAUM, BA
SHMON@RISEFLARCHITECTURE.COM
908.674.8717

FL OFFICE
241 37TH STREET SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENEBAUM, BA
SHMON@RISEFLARCHITECTURE.COM
908.674.8717

STRUCTURAL CONSULTANT

JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT

CIVIL CONSULTANT

DESIGNER CONSULTANT

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT

PROJECT NAME / LOCATION
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE
REBAR LAP & DEVELOPMENT SCHEDULES

EXAMINER'S SIGNATURE

PROFESSIONAL SEAL

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:38:10 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted
DATE:
PROJECT NO: 2318_07

DRAWING NO: S004

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:04 AM

FOOTING SCHEDULE 2,500 P.S.F. BEARING CAPACITY

MK	SIZE LENGTH	SIZE WIDTH	DEPTH (INCHES)	BOTT. REINF. EA-EACH WAY L-LONG S-SHORT	REMARKS
EMF1.0	11'-10"	9'-7"	12"	#5 AT 12" O.C. (E.W.)	(TOP AND BOTTOM)
EMF2.0	12'-6"	13'-4"	18"	#5 AT 12" O.C. (E.W.)	(TOP AND BOTTOM)
F5.5	5'-6"	5'-6"	12"	5 - #5	
F6.0	6'-0"	6'-0"	15"	7 - #5	
F6.5	6'-6"	6'-6"	15"	7 - #5	
F7.5	7'-6"	7'-6"	18"	10 - #5	
F8.0	8'-0"	8'-0"	18"	11 - #5	
F10.0	10'-0"	10'-0"	21"	9 - #7	
F10.5	10'-6"	10'-6"	21"	9 - #7	
F11.0	11'-0"	11'-0"	24"	10 - #7	

NOTES:
1. SPREAD FOOTING SHALL BE NORMAL WEIGHT CONCRETE (f'c = 4,000 psi AT 28 DAYS).
2. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL CONFORM WITH THE "MINIMUM CONCRETE COVER FOR REINFORCING BAR" TABLE PROVIDED. (SEE ACI 318 SECTION 7.7 FOR CONDITIONS NOT NOTED).

STRIP FOOTING SCHEDULE 2,500 P.S.F. BEARING CAPACITY

MK	SIZE WIDTH (INCHES)	SIZE DEPTH (INCHES)	REINFORCEMENT LONGITUDINAL	REINFORCEMENT TRANSVERSE
SF2.0	24"	12"	(3) #5 BARS	#5 BARS 36" O.C.
SF2.5	30"	12"	(3) #5 BARS	#5 BARS 36" O.C.

NOTES:
1. STRIP FOOTING SHALL BE NORMAL WEIGHT CONCRETE (f'c = 4,000 psi AT 28 DAYS).
2. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL CONFORM WITH THE "MINIMUM CONCRETE COVER FOR REINFORCING BAR" TABLE PROVIDED. (SEE ACI 318 SECTION 7.7 FOR CONDITIONS NOT NOTED).

SLAB ON GRADE CONSTRUCTION SCHEDULE

MARK	AREA	f'c (psi)	THICKNESS	REINFORCEMENT
①	INT. SOG	4,000	6"	WWF 6x6-W1.4xW1.4 (SEE NOTE 5)

NOTES:
1. ① INDICATES CONCRETE SLAB ON GRADE PLACED OVER A VAPOR BARRIER ON 6" OF CRUSHED STONE
2. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL CONFORM WITH THE "MINIMUM CONCRETE COVER FOR REINFORCING BAR" TABLE PROVIDED. (SEE ACI 318 SECTION 7.7 FOR CONDITIONS NOT NOTED).
3. ANY EXTERIOR STAIRS, RAMPS, OR ELEVATED PLATFORMS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000PSI CONCRETE.
4. CONTROL JOINTS IN SLAB ON GRADE SHALL BE LOCATED AND SPACED IN ACCORDANCE WITH TYPICAL CONTROL JOINT DETAIL ON S502.
5. CONTRACTOR MAY SUBSTITUTE 1.5 POUNDS OF FIBROUS REINFORCEMENT PER CUBIC YARD OF CONCRETE IN LIEU OF WWF.
6. REFERENCE TYPICAL CONCRETE SLAB DETAILS ON S502 FOR ADDITIONAL INFORMATION.

GRADE BEAM SCHEDULE

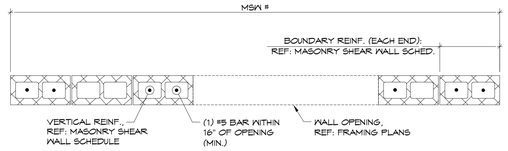
MK	SIZE WIDTH (INCHES)	SIZE DEPTH (INCHES)	LONGITUDINAL REINFORCEMENT BOTTOM BARS	LONGITUDINAL REINFORCEMENT TOP BARS CONT.	LONGITUDINAL REINFORCEMENT ADD'L BARS	STIRRUPS SIZE	STIRRUPS SPACING MID	STIRRUPS SPACING ENDS	# OF LEGS	REMARKS
GB3618-1	36"	18"	(4) #9	(4) #9	-	#4	7"	7"	4L	

NOTE:
1. ALL CONCRETE GRADE BEAMS SHALL BE NORMAL WEIGHT CONCRETE (f'c=4,000PSI AT 28 DAYS).
2. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL CONFORM WITH THE "MINIMUM CONCRETE COVER FOR REINFORCING BAR" TABLE PROVIDED. (SEE ACI 318 SECTION 7.7 FOR CONDITIONS NOT NOTED).
3. REINFORCING DEVELOPMENT AND LAP SPLICE LENGTH, REFERENCE TABLE ON S004.
4. 'END' DENOTES BEGINNING AND END THIRD OF BEAM SPAN FROM SUPPORT TO SUPPORT.
5. 'MID' DENOTES MIDDLE THIRD OF BEAM SPAN FROM SUPPORT TO SUPPORT.

MASONRY WALL SCHEDULE

MK	THICKNESS	f'm	VERTICAL REINF.	HORIZ. REINF.	BOUNDARY REINF.	BOND BEAM SPACING (SEE NOTE 7)
MW8.1	8"	2,000psi	#5 BARS AT 32" O.C.	(SEE NOTE 4)	N/A	N/A
MSW8.1[2,3]	8"	2,000psi	#5 BARS AT 32" O.C.	(SEE NOTE 4)	(2) #5 BARS	10'-0" O.C. (MAX.)
MN8.1[11]	8"	2,000psi	#5 BARS AT 32" O.C.	(SEE NOTE 4)	N/A	10'-0" O.C. (MAX.)
MN12.1[11]	12"	2,000psi	#5 BARS AT 32" O.C.	(SEE NOTE 4)	(2) #5 BARS	10'-0" O.C. (MAX.)

NOTES:
1. MASONRY WALLS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF f'm = 2000psi U.N.O.. FILL ALL REINFORCED CELLS SOLID WITH 3000psi GROUT.
2. "MSW" DENOTES MASONRY SHEAR WALL. FILL ALL REINFORCED CELLS SOLID WITH 3,000psi GROUT. REFERENCE TYPICAL SHEAR WALL DETAIL ON S205 FOR ADDITIONAL INFORMATION.
3. FILL ALL CELLS BELOW GRADE WITHH 3,000psi GROUT.
4. WALL THICKNESS AND REINFORCEMENT SHALL APPLY TO ALL LEVELS OF WALL ABOVE POINT OF DESIGNATION OR UNTIL NEXT WALL DESIGNATION.
5. PROVIDE HORIZ. REINFORCEMENT (9 ga. GALVANIZED TRUSS TYPE) AT 16" O.C.
6. VERTICAL REINFORCEMENT BARS SHALL BE PLACED AT EACH WALL END AND CORNER THEN SPACED PER INDICATED SPACING.
7. PROVIDE 16" DEEP BOND BEAMS AT EACH FLOOR LEVEL w/ (1) #6 BARS CONTINUOUS. PROVIDE 16" DEEP BOND BEAMS BETWEEN FLOORS WHERE WALL IS GREATER THAN 10ft w/ (1) #6 BAR (TOP & BOTTOM) CONTINUOUS. ALL BOND BEAMS SHALL HAVE KNOCK OUT BLOCKS AND SHALL BE FILLED SOLID WITH 3000PSI GROUT, REFERENCE TYPICAL DETAIL ON S531 FOR ADDITIONAL INFORMATION.
8. CONTRACTOR SHALL MAINTAIN RUNNING BOND THROUGHOUT WALLS AND CORNERS.
9. REFERENCE FRAMING PLANS FOR LINTELS, STEEL BEAMS, BEARING PLATES, AND LEDGER CONNECTION LOCATIONS.
10. REFERENCE TYPICAL CONCRETE MASONRY WALL CONSTRUCTION DETAILS ON S531 FOR ADDITIONAL INFORMATION.
11. REFERENCE TYPICAL MASONRY SHEAR WALL DETAILS FOR ADDITIONAL INFORMATION.



TYPICAL MASONRY SHEAR WALL DETAIL

CONCRETE WALL SCHEDULE

MK	THICKNESS	NO OF CURTAINS	VERTICAL REINF.	HORIZONTAL REINF.	REMARKS
CW12.1	1'-0"	2	SEE NOTE 5	#5 BARS AT 18" O.C.	

NOTES:
1. ALL CONCRETE WALLS SHALL BE 5,000PSI CONCRETE.
2. WALL THICKNESS AND REINFORCEMENT SHALL APPLY TO ALL LEVEL OF WALL ABOVE POINT OF DESIGNATION OR UNTIL NEXT WALL DESIGNATION.
3. REFERENCE TYPICAL CONCRETE WALL CONSTRUCTION DETAILS ON S516 FOR ADDITIONAL INFORMATION.
4. (C) - CENTERED / (OF) - OUTSIDE FACE / (IF) - INSIDE FACE.
5. ALL CONCRETE WALLS BELOW MASONRY WALL SHALL MATCH REINFORCEMENT OF MASONRY WALL ABOVE.

LOAD BEARING WALL LINTEL SCHEDULE

MK	TYPE	SIZE (W)x(H) (IN)	REINFORCEMENT	REMARKS
ML1	MASONRY	8x16	(1) #6 BARS (TOP & BOTTOM)	N/A
ML2	MASONRY	8x24	(1) #6 BARS (TOP & BOTTOM)	N/A

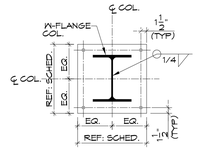
NOTES:
1. REFERENCE S531 FOR ALL APPLICABLE TYPICAL DETAILS.

STEEL COLUMN SCHEDULE

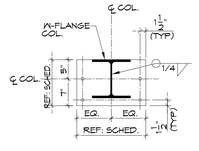
MK	COLUMN SIZE
C1	W10X49
C2	W12X53

BASE PLATE SCHEDULE

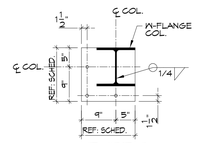
MK	LENGTH	WIDTH	THICKNESS	ANCHOR BOLTS	EMBEDMENT	BASE PLATE TYPE
BP1	18"	18"	1/2"	(4) 3/4" DIA. F1554	12"	TYPE 1
BP2	18"	12"	1/2"	(4) 3/4" DIA. F1554	12"	TYPE 2
BP3	14"	14"	1/2"	(3) 3/4" DIA. F1554	12"	TYPE 3



TYPE 1



TYPE 2



TYPE 3

METAL DECK SCHEDULE

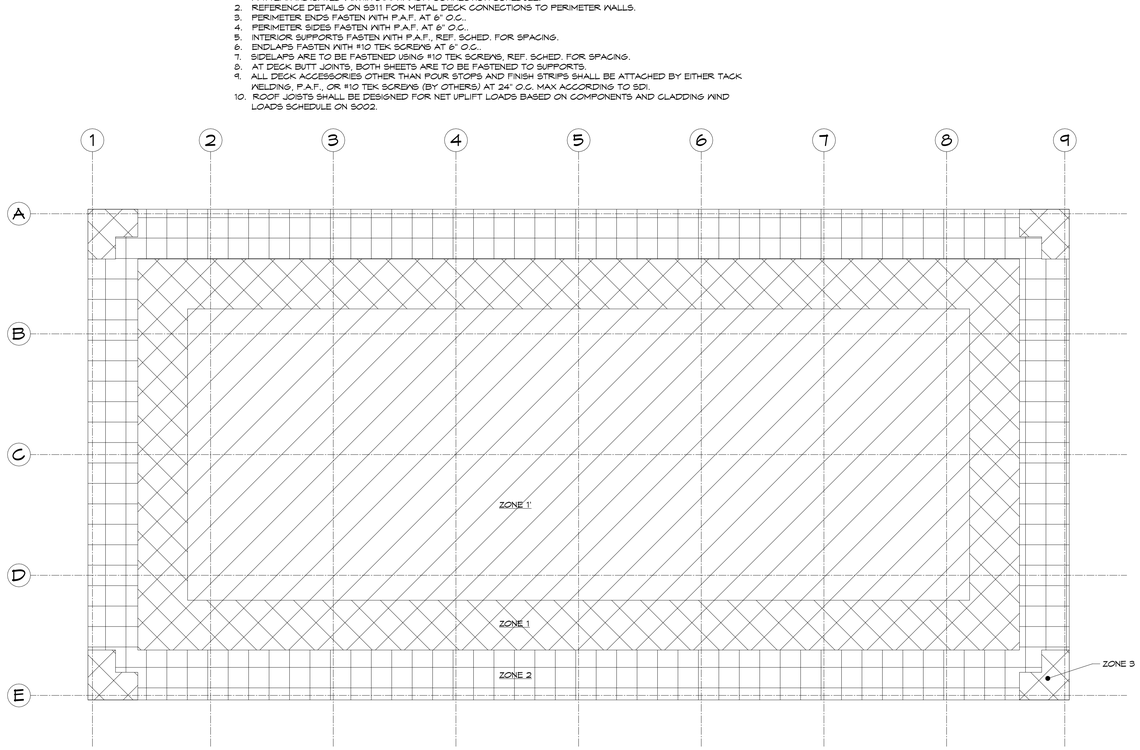
MK	DECK SIZE	SLAB THICKNESS (IN) NORMAL WEIGHT	TOTAL THICKNESS (IN)	CONCRETE f'c (psi) (AT 28 DAYS)	SLAB REINFORCEMENT (SEE NOTE 1 & 3)	FLUTE REINFORCEMENT
1	3VLI20	3-1/2"	6-1/2"	3,500	WWF 6x6-W2.9xW2.9	N/A
2	1.5B18	N/A	1-1/2"	N/A	N/A	N/A

NOTES:
1. — ⑤ — INDICATES DIRECTION OF FRAMING.
2. REFERENCE S525 FOR ALL APPLICABLE TYPICAL DETAILS.
3. REINFORCE WITH WWF 6x6-W1.4xW1.4 AND #4 x 8'-0" AT 12" O.C. OVER GIRDERS.

DIAPHRAGM CONNECTION SCHEDULE

MK	CONNECTION PATTERN AT SUPPORTS	CONNECTION AT SIDELAPS
ALL ZONES	HILTI X-ENP19 P.A.F. AT 36/4	#10 TEK SCREWS AT 12" O.C. (MAX.)

NOTES:
1. TYPICAL METAL DECK CONNECTION SHALL CONSIST OF HILTI X-ENP19 P.A.F. IN ACCORDANCE WITH CONNECTION PATTERN INDICATED WITHIN DIAPHRAGM CONNECTION SCHEDULE.
2. REFERENCE DETAILS ON S311 FOR METAL DECK CONNECTIONS TO PERIMETER WALLS.
3. PERIMETER ENDS FASTEN WITH P.A.F. AT 6" O.C..
4. PERIMETER SIDES FASTEN WITH P.A.F. AT 6" O.C..
5. INTERIOR SUPPORTS FASTEN WITH P.A.F., REF. SCHED. FOR SPACING.
6. ENDLAPS FASTEN WITH #10 TEK SCREWS AT 6" O.C..
7. SIDELAPS ARE TO BE FASTENED USING #10 TEK SCREWS, REF. SCHED. FOR SPACING.
8. AT DECK BUTT JOINTS, BOTH SHEETS ARE TO BE FASTENED TO SUPPORTS.
9. ALL DECK ACCESSORIES OTHER THAN POUR STOPS AND FINISH STRIPS SHALL BE ATTACHED BY EITHER TACK WELDING, P.A.F., OR #10 TEK SCREWS (BY OTHERS) AT 24" O.C. MAX ACCORDING TO SDI.
10. ROOF JOISTS SHALL BE DESIGNED FOR NET UPLIFT LOADS BASED ON COMPONENTS AND CLADDING WIND LOADS SCHEDULE ON S002.



ROOF DIAPHRAGM CONNECTION PLAN

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

SCHEDULES

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:38:32 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

PROJECT NO. 2318_07

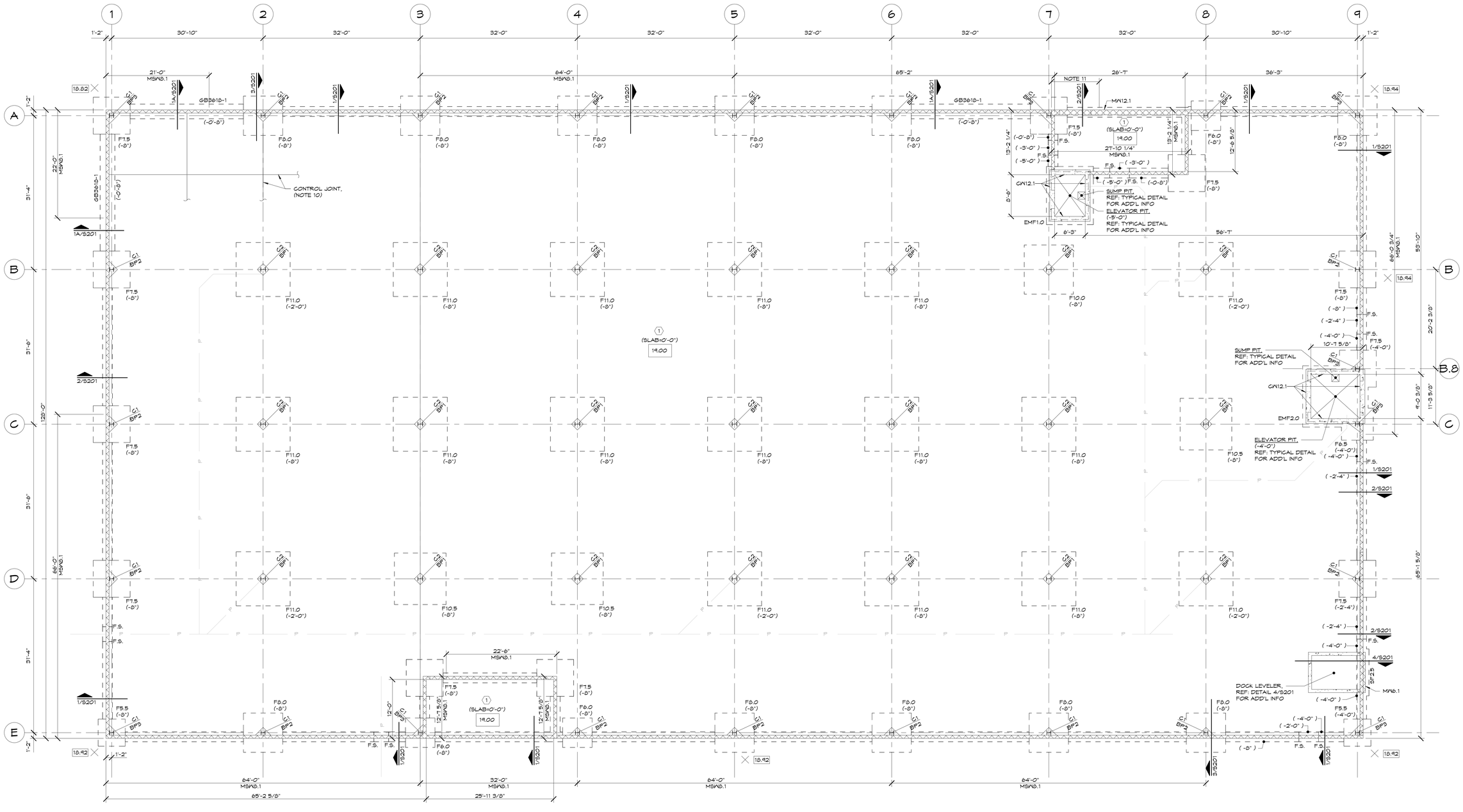
S005

DRAWING NO.

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:07 AM



ALL STRIP FOOTING SHALL BE "SF2.0" AT ELEVATION (-8") UNLESS OTHERWISE NOTED.

FOOTING & FOUNDATION NOTE:
FOOTING AND FOUNDATIONS WERE DESIGN BASED ON AN ASSUMED ALLOWABLE SOIL BEARING PRESSURE OF 2,500PSF. A GEOTECHNICAL ENGINEER SHALL BE CONTRACTED TO VERIFY EXISTING SOIL CONDITIONS DURING CONSTRUCTION TO CONFIRM BEARING PRESSURE AND COMPACTION OF SUB-GRADE. IF ASSUMED BEARING CAPACITY IS NOT OBTAINABLE, ENGINEER OF RECORD SHALL BE NOTIFIED TO REEVALUATE THE FOUNDATION SYSTEM.

VERTICAL DOWELS SHALL BE PROVIDED INTO CONCRETE FOOTINGS FOR ALL MASONRY WALL REINFORCEMENT, INCLUDING BOUNDARY BARS. REFERENCE MASONRY WALL SCHEDULE FOR REQUIRED BAR SIZE.

SOIL COMPACTION NOTE:
ALL AREAS DESIGNATED "COMPACTED FILL" ON PLAN SHALL BE VIRGIN SOIL OR CONTROL COMPACTED FILL. FILL SHALL BE PLACED IN 6" LIFTS AND COMPACTED WITH PLATE TAMPER.

ALL MASONRY WALLS SHALL BE "MA6.1" UNLESS OTHERWISE NOTED.

REFERENCE CONTROL JOINT PLAN ON S101A FOR MASONRY WALL CONTROL JOINT LOCATIONS.

FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES:

1. TOP OF CONCRETE SLAB ELEVATION IS AT (19.00) AND IS SET AS DATUM 0'-0".
2. TOP OF SLAB IS AT (0'-0") UNLESS NOTED THUS (SLAB=+x'-x") ON PLAN REFERENCED FROM THE BUILDING'S DATUM.
3. FOOTINGS SHALL BEAR ON VIRGIN SOIL OR CONTROLLED COMPACTED FILL HAVING A MINIMUM BEARING CAPACITY OF 1.25 TONS PER SQUARE FOOT.
4. Fx.x INDICATES FOOTING TYPE, SEE FOOTING SCHEDULE FOR SIZE AND REINFORCING.
5. TOP OF FOOTING ELEVATIONS ARE SHOWN THUS (x'-x") ON PLAN AND ARE REFERENCED FROM THE BUILDING'S DATUM.
6. ALL COLUMN FOOTINGS SHALL BE CENTERED UNDER COLUMN CENTERLINES UNLESS NOTED OTHERWISE ON PLAN.
7. PROVIDE THICKENED SLAB DETAIL AT INTERIOR MASONRY PARTITIONS WHERE FOOTINGS ARE NOT SHOWN ON PLAN, SEE DETAIL ON TYPICAL DETAIL SHEETS.
8. F.S. - DENOTES FOOTING STEP. REFERENCE TYPICAL DETAIL.
9. AT ALL LOCATIONS WHERE PLUMBING LINES EXIT THE BUILDING, DEEPEN FOOTINGS AS REQUIRED TO ALLOW 6" (MIN) CLEARANCE BETWEEN PIPE AND FOOTING.
10. CONTROL JOINTS IN SLAB ON GRADE SHALL BE LOCATED AND SPACED IN ACCORDANCE WITH TYPICAL CONTROL JOINT DETAIL.
11. PROVIDE (3) ADDITIONAL #5 BARS AT 8" O.C. AT TWO-STORY WINDOW JAMB WHERE INDICATED ON PLAN. DOWELS TO MATCH REINFORCEMENT SHALL BE PROVIDED INTO CONCRETE FOOTINGS.

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD, SUITE 400
LAKEWOOD, NJ 08701
SHMOM GREENBAUM, PA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENBAUM, PA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
FOUNDATION PLAN

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:38:51 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

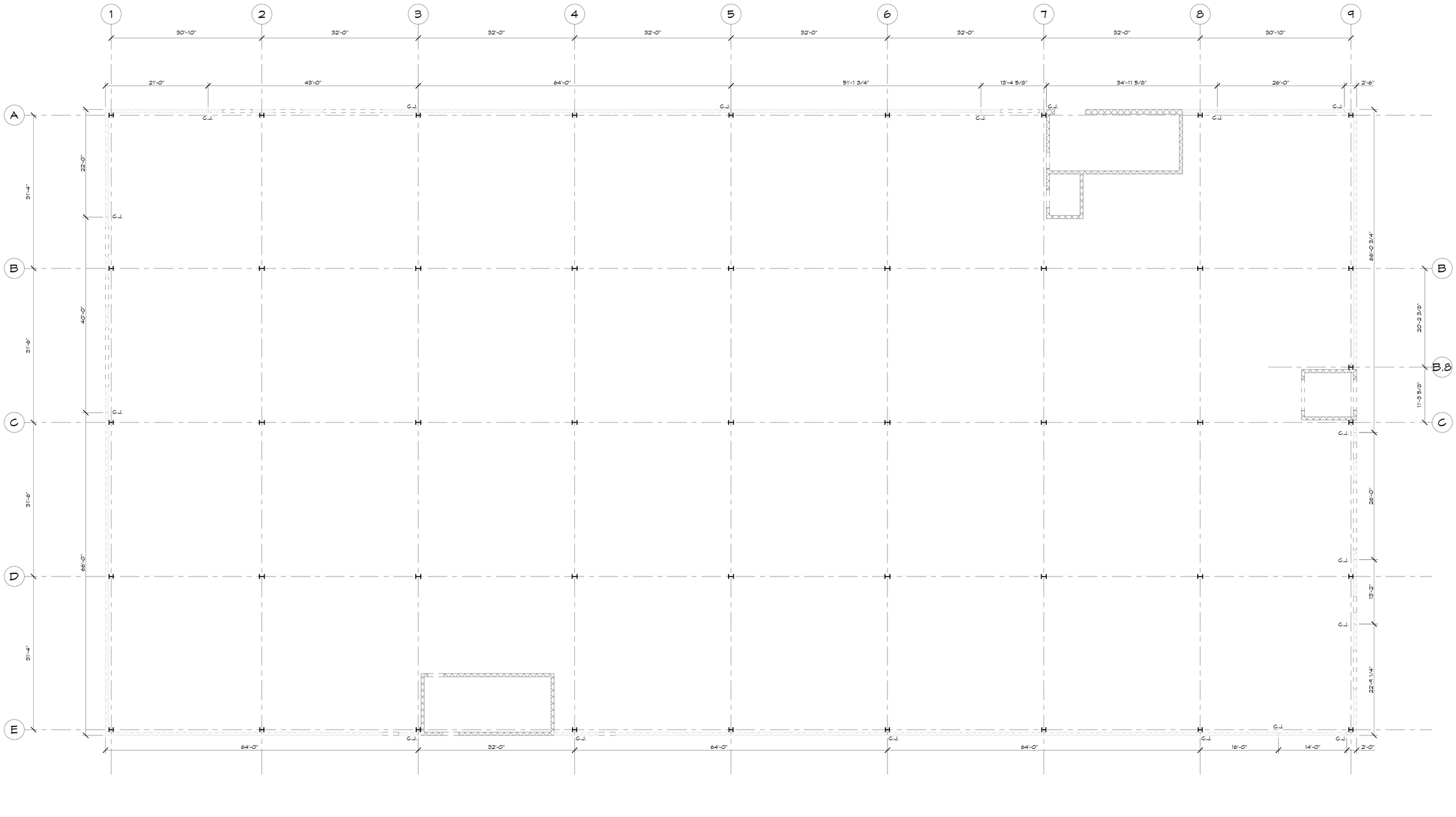
SCALE: As Noted
DATE:
PROJECT NO: 2318_07

DRAWING NO:
S101

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:11 AM



CONTROL JOINT PLAN

SCALE: 1/8"=1'-0"

NOTES:

1. CONTROL JOINTS SHOWN ON PLAN ARE FOR LOAD BEARING AND SHEAR WALLS ONLY. NON LOAD BEARING MASONRY WALLS SHALL BE INSTALLED WITH CONTROL JOINTS. REFERENCE TYPICAL DETAILS ON S531 FOR LOCATION REQUIREMENTS.
2. REFERENCE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS FOR BRICK FACADES.
3. C.J. DENOTES MASONRY CONTROL JOINT. REFERENCE TYPICAL DETAILS ON S531 FOR REQUIRED CONSTRUCTION.

RISE | FL
architecture

ARCHITECT:

NYC OFFICE:
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE:
36 AIRPORT RD, SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENEBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE:
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENEBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

CONTROL JOINT PLAN

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:40:17 -04'00'

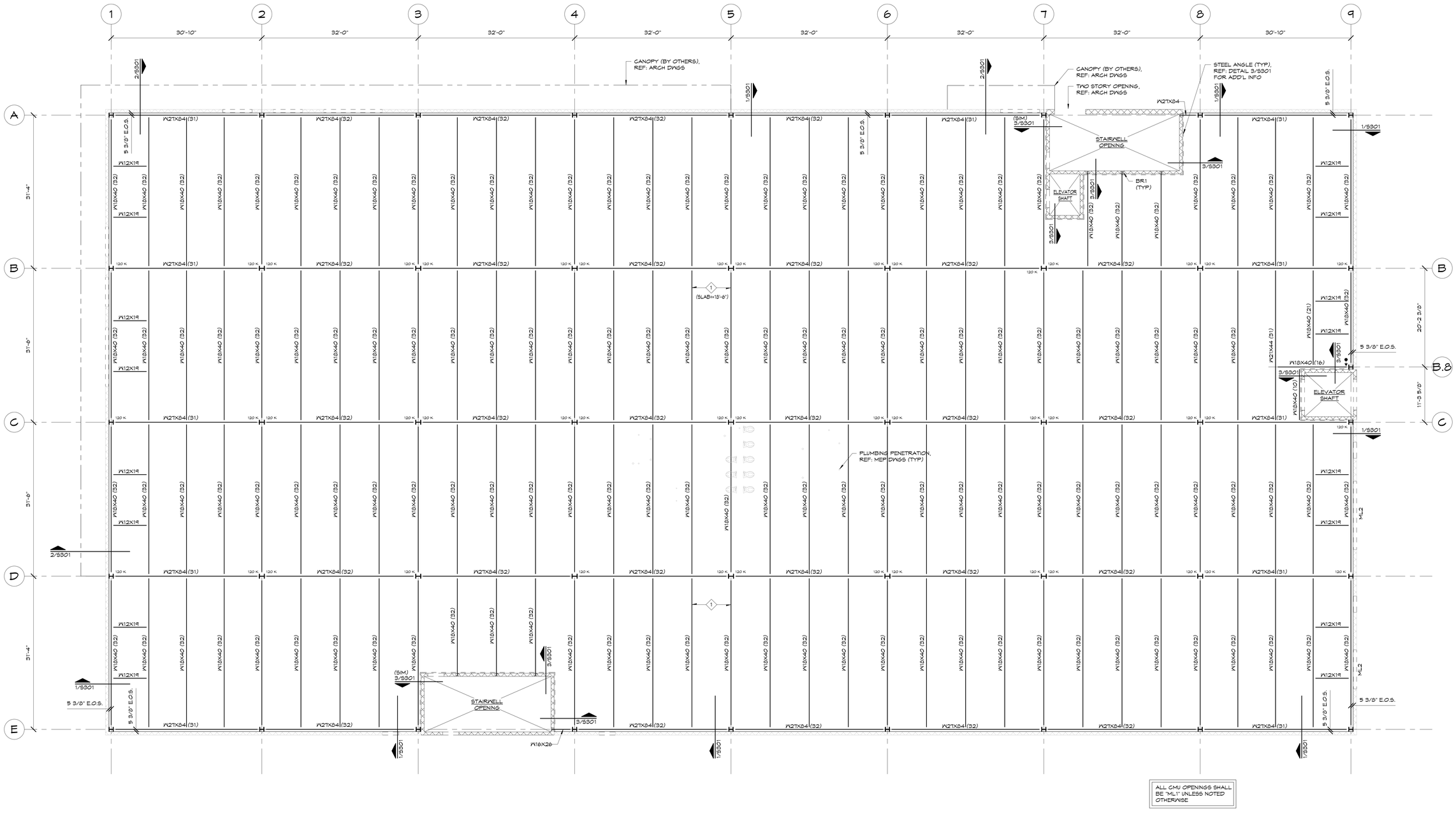
JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE:	As Noted
DATE:	
PROJECT NO:	2318_07
DRAWING NO:	S101A

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:11 AM



2ND FLOOR FRAMING PLAN

SCALE: 1/8"=1'-0"

NOTES:

1. TOP OF CONCRETE SLAB IS NOTED THUS (SLAB=+x-x") ON PLAN AND REFERENCED FROM BUILDING'S DATUM.
2. TOP OF STEEL ELEV. IS 6-1/2" BELOW TOP OF SLAB UNLESS NOTED THUS (T.O.S. +x"-x") ON PLAN AND IS REFERENCED FROM THE BUILDING'S DATUM.
3. STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 (UNO).
4. BEAMS AND JOISTS SHOWN ON PLAN ARE EQUALLY SPACED BETWEEN GOLUMN CENTERLINES UNLESS OTHERWISE NOTED ON PLAN.
5. (...) INDICATES NUMBER OF 3/4" DIAMETER HEADED STUDS MACHINE WELDED TO BEAM TOP FLANGE. REFERENCE TYPICAL COMPOSITE SLAB CONSTRUCTION DETAIL AND STEEL GENERAL NOTES FOR ADDITIONAL INFORMATION.
6. COLUMN SCHEDULE SEE DRAWING S004.
7. BR# INDICATES STEEL BEARING PLATE. REFERENCE SCHEDULE AND TYPICAL DETAILS FOR ADDITIONAL INFORMATION.

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)

FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

2ND FLOOR FRAMING PLAN

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino

Digitally signed by Jason Tarantino
Date: 2024.06.24 11:40:44 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE:	As Noted
DATE:	
PROJECT NO:	2318_07

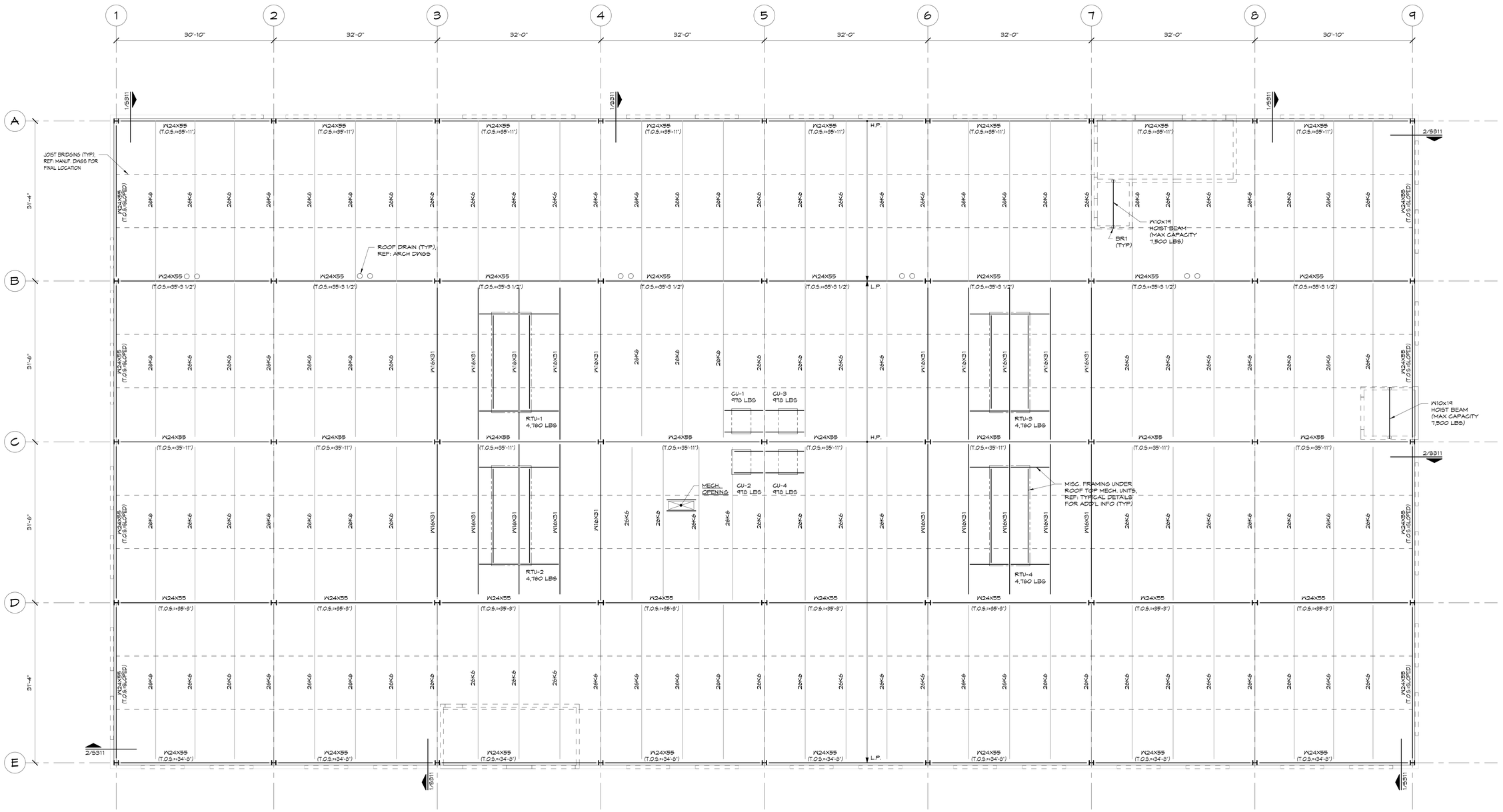
DRAWING NO:

S102

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:15 AM



ALL CMU OPENINGS SHALL HAVE CONTINUOUS BOND BEAM ABOVE AND BELOW OPENINGS U.N.O. REFERENCE MASONRY WALL SCHEDULE FOR SIZE & REINFORCEMENT.

ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES:

1. TOP OF STEEL ELEVATION SHOWN THUS (T.O.S.=X'-X") ON PLAN AND IS REFERENCED FROM THE BUILDING'S DATUM.
2. BEAMS AND JOISTS SHOWN ON PLAN ARE SPACED EQUALLY BETWEEN COLUMN CENTERLINES UNLESS OTHERWISE NOTED ON PLAN.
3. BR= INDICATES STEEL BEARING PLATE. SEE SCHEDULE AND TYPICAL DETAIL FOR ADDITIONAL INFORMATION.
4. ROOF JOISTS SHALL BE DESIGNED FOR NET UPLIFT LOADS BASED ON COMPONENTS AND CLADDING WIND LOADS SCHEDULE ON S002.

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD, SUITE 400
LAKEWOOD, NJ 08701
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

ROOF FRAMING PLAN

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino

Digitally signed by Jason Tarantino
Date: 2024.06.24 11:41:02 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

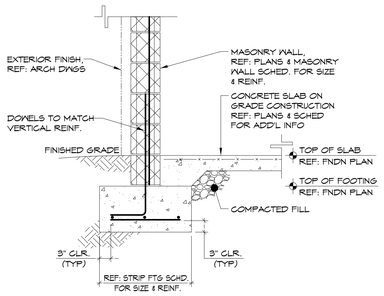
PROJECT NO: 2318_07

DRAWING NO: S103

BARCODE

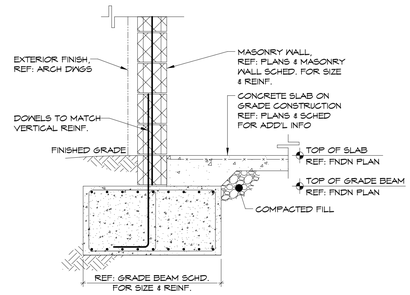
THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:19 AM



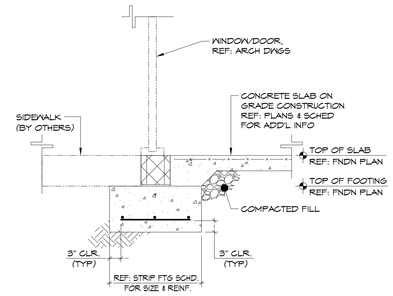
1 FOUNDATION WALL DETAIL

SCALE: 3/4"=1'-0"



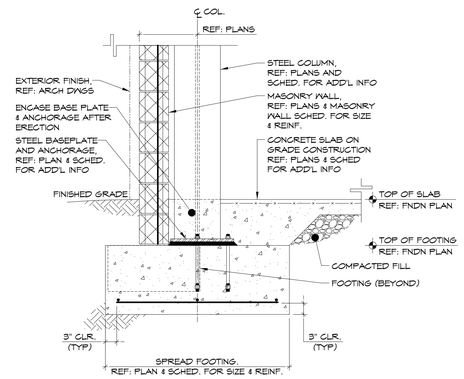
1A FOUNDATION WALL DETAIL

SCALE: 3/4"=1'-0"



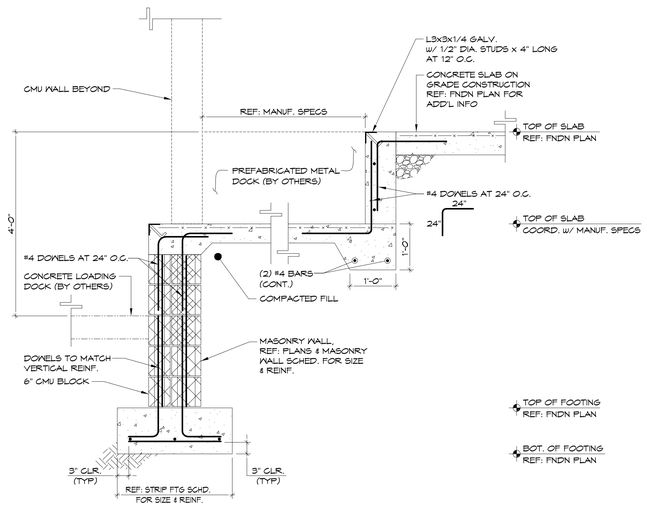
2 FOUNDATION WALL DETAIL

SCALE: 3/4"=1'-0"



3 COLUMN FOUNDATION DETAIL AT CMU WALL

SCALE: 3/4"=1'-0"



4 DOCK LEVELER PIT DETAIL

SCALE: 3/4"=1'-0"

RISE | FL
architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

FOUNDATION DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino

Digitally signed by Jason Tarantino
Date: 2024.06.24 11:41:22 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

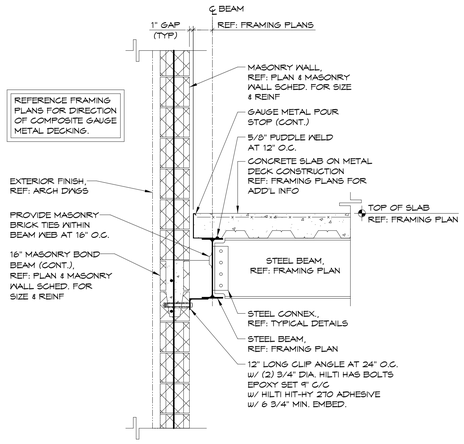
PROJECT NO: 2318_07

DRAWING NO: S201

BARCODE

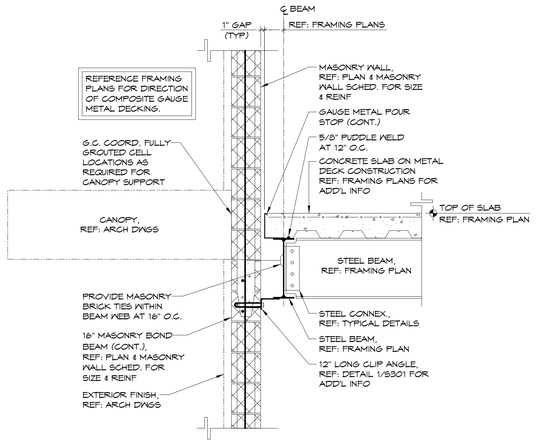
THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:20 AM



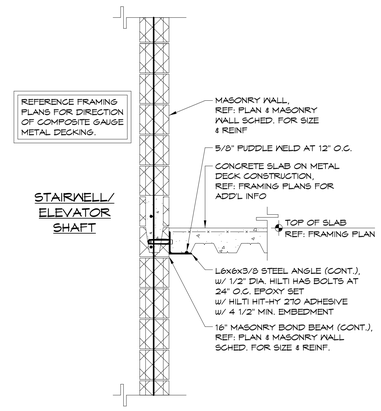
1 EXTERIOR WALL FRAMING DETAIL

SCALE: 3/4"=1'-0"



2 CANOPY FRAMING DETAIL

SCALE: 3/4"=1'-0"



3 FRAMING AT INTERIOR SHAFT

SCALE: 3/4"=1'-0"

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

FRAMING DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino

Digitally signed by Jason Tarantino
Date: 2024.06.24 11:41:48 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

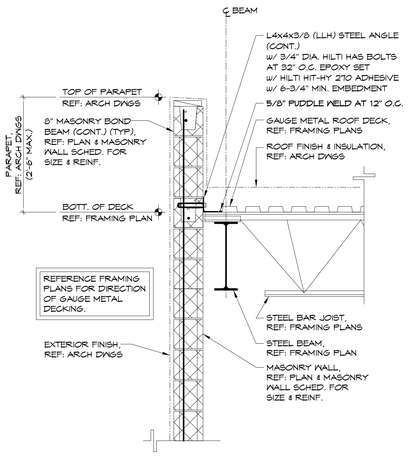
PROJECT NO. 2318_07

DRAWING NO.: S301

BARCODE:

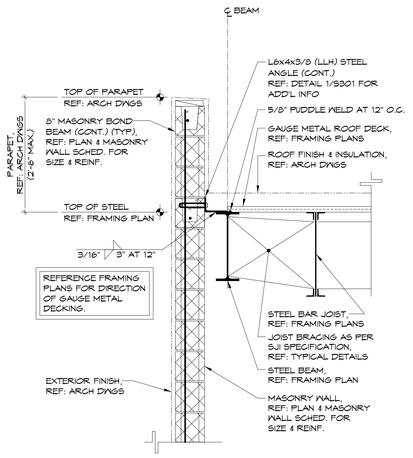
THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:20 AM



1 ROOF FRAMING DETAIL AT CMU WALL

SCALE: 3/4"=1'-0"



2 ROOF FRAMING DETAIL AT CMU WALL

SCALE: 3/4"=1'-0"

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENEBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENEBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

ROOF FRAMING DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino

Digitally signed by Jason Tarantino
Date: 2024.06.24 11:42:10 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

PROJECT NO. 2318_07

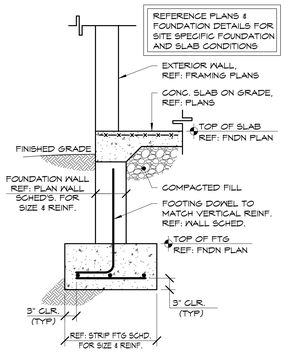
DRAWING NO.

S311

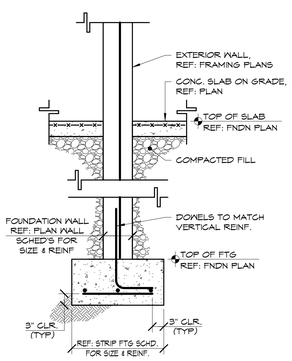
BARCODE:

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

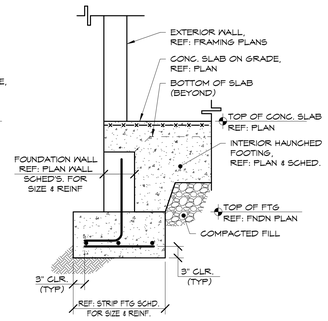
6/24/2024 8:16:21 AM



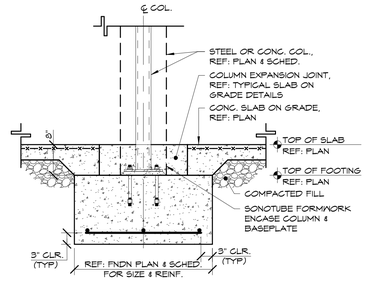
EXTERIOR W/ S.O.G.



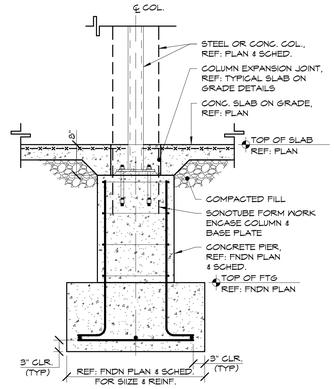
EXTERIOR W/ S.O.G.
TYPICAL STRIP FOOTING DETAILS



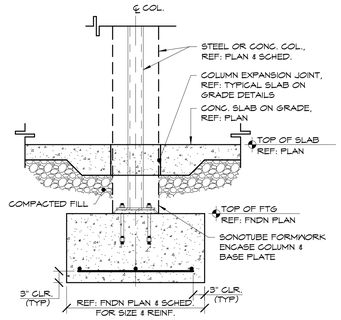
EXTERIOR W/ HAUNCHED INTERIOR FOOTING



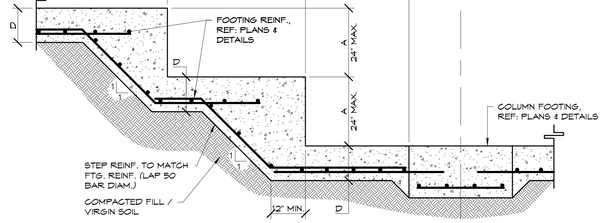
INTERIOR COLUMN



COLUMN W/ PIER
TYPICAL SPREAD FOOTING DETAILS



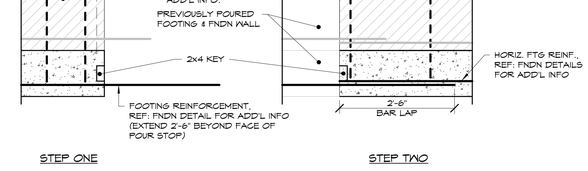
TYPICAL SPREAD FOOTING WITH HAUNCHED SLAB DROPPED COLUMN W/ ENGASEMENT



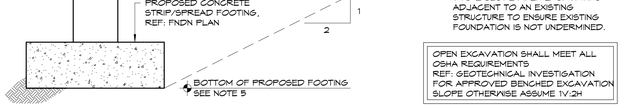
PLAN

ELEVATION

TYPICAL FOOTING STEP DETAIL

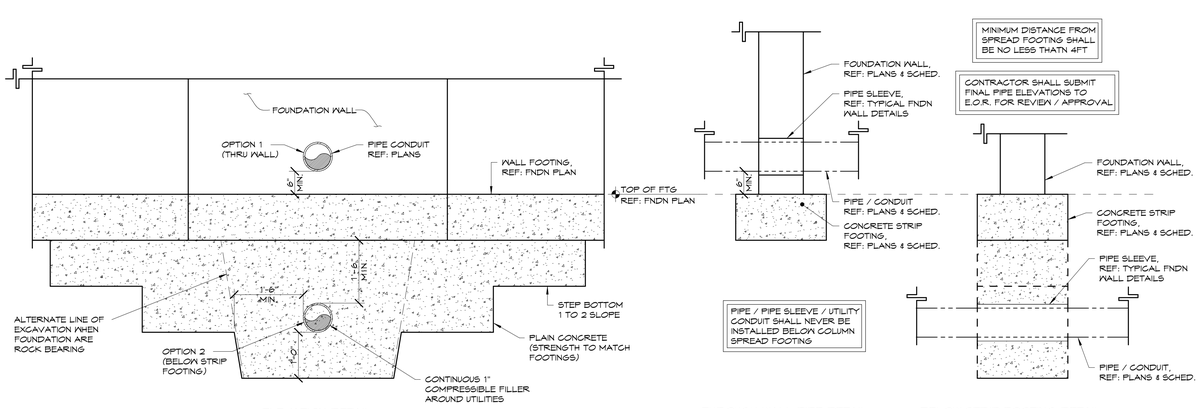


CONCRETE STRIP FOOTING CONSTRUCTION JOINT DETAIL

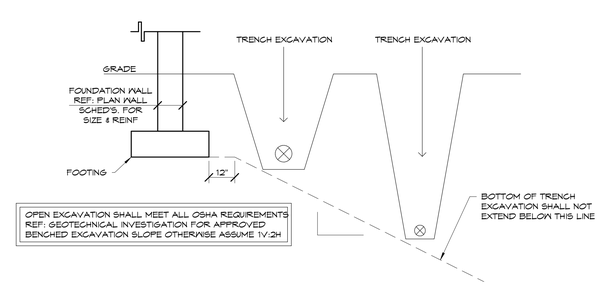


NOTES:
1. PRE-CONDITION SURVEY AND MONITOR OF EXISTING BUILDING STRUCTURE IS RECOMMENDED DURING EXCAVATION TO ENSURE ANY MOVEMENT / SETTLEMENT WITHIN BUILDING IS IDENTIFIED.
2. CONTRACTOR SHALL USE EXTREME CAUTION WHEN EXCAVATING ADJACENT TO EXISTING STRUCTURE & FOOTING AS TO ENSURE EXISTING FOUNDATION IS NOT DAMAGED.
3. CONTRACTOR SHALL PROVIDE WEATHER PROTECTION TO EXISTING FOOTING AND SOIL TO ENSURE EROSION & UNDERMINING DOES NOT OCCUR DURING THE EXCAVATION AND CONSTRUCTION.
4. IF 1 TO 2 SLOPE IS NOT OBTAINABLE, CONTRACTOR SHALL BE REQUIRED TO UNDERPIN EXISTING FOOTING TO A DEPTH THAT WILL MAINTAIN THE EXISTING FOOTINGS BEARING CAPACITY. E.O.R. SHALL BE NOTIFIED TO PROVIDE DETAILED UNDERPINNING / SEQUENCING PLAN.
5. REFERENCE FOUNDATION PLAN FOR PROPOSED BOTTOM OF FOOTING. SUB-GRADE SHALL BE FIELD VERIFIED BY LICENSED GEOTECHNICAL ENGINEER TO CONFIRM SOIL BEARING CAPACITY AT PROPOSED FOOTING ELEVATION.

TYPICAL FOOTING EXCAVATION ADJACENT TO EXISTING STRUCTURE



TYPICAL PIPE / UTILITY CROSSING AT STRIP FOOTING



TYPICAL UNDERGROUND PIPE EXCAVATION DETAIL

RISE | FL architecture
ARCHITECT:
NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746
NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717
FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717
STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com
MEP CONSULTANT:
CIVIL CONSULTANT:
DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
TYPICAL SPREAD FOOTING DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:42:42 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

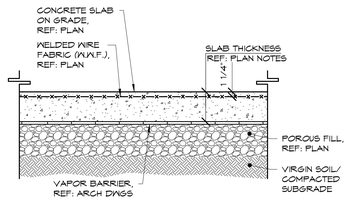
SCALE: As Noted
DATE:
PROJECT NO. 2318_07

DRAWING NO.
S501

BARCODE

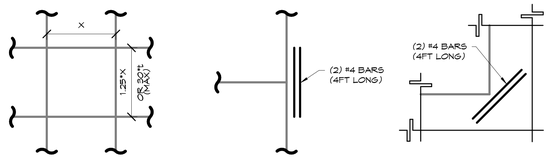
THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:24 AM

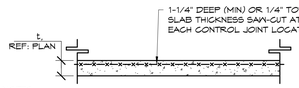


FIBROUS REINFORCEMENT ALTERNATE:
CONTRACTOR MAY SUBSTITUTE SIKA FIBERSMESH 300 REINFORCEMENT OR APPROVED EQUAL TO CONCRETE MIX IN LIEU OF REINFORCEMENT. (NOT RECOMMENDED FOR POLISH CONCRETE).

TYPICAL CONCRETE SLAB ON GRADE



TYPICAL CONTROL JOINT LAYOUT | DISCONTINUOUS JOINTS | REENTRANT CORNERS

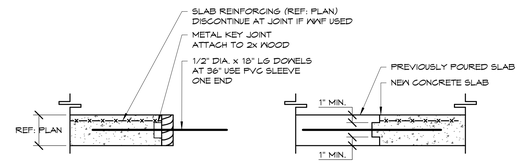


NOTES:
1. MAXIMUM JOINT SPACING SHALL BE 30x SLAB THICKNESS (t) U.N.O.
2. PANELS SHALL BE SQUARE SHAPED. IF PANELS ARE UNABLE TO BE SQUARE, LENGTH OF LONG EDGE OF PANEL SHALL NOT EXCEED 1.25x LENGTH OF SHORT EDGE
3. CONTRACTOR MAY PROVIDE PREFORMED CONTROL JOINT SUCH AS "STRESSLOCK" OR APPROVED EQUAL.
4. UPON CONTRACTOR'S REQUEST, FORMAL CONTROL JOINT LAYOUT CAN BE PROVIDED.

RECOMMENDED CONTROL JOINT SPACING

SLAB THICKNESS (t) (IN.)	MAX SPACING (FT.)
4	10
5	12
6	15
7	17
8	20

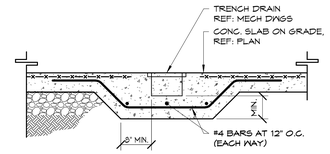
TYPICAL CONTROL JOINT DETAIL



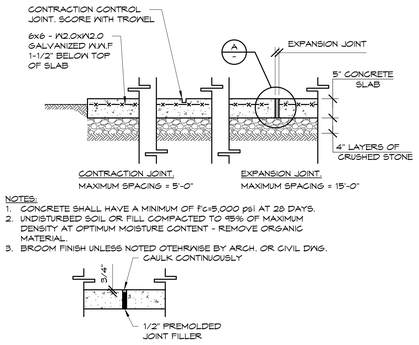
STEP ONE

STEP TWO
REMOVE WOOD FORMS FOR SECOND POUR.

TYPICAL SLAB ON GRADE CONSTRUCTION JOINT



TYPICAL SLAB ON GRADE AT TRENCH DRAIN

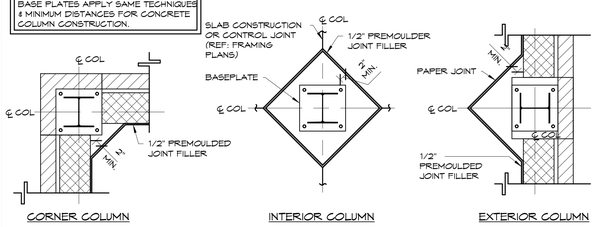


NOTES:
1. CONCRETE SHALL HAVE A MINIMUM OF f'c=3,000 psi AT 28 DAYS.
2. UNDISTURBED SOIL OR FILL COMPACTED TO 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT - REMOVE ORGANIC MATERIAL.
3. BROOM FINISH UNLESS NOTED OTEHRWISE BY ARCH. OR CIVIL DWG.

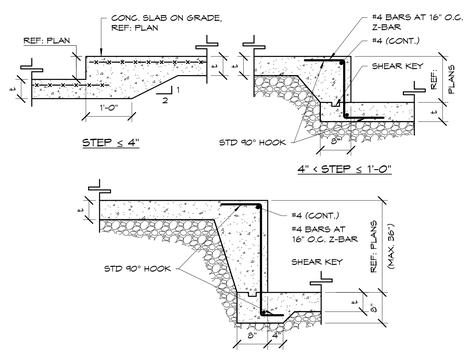
A DETAIL
SCALE: NOT TO SCALE

TYPICAL EXTERIOR CONCRETE SIDEWALK DETAILS

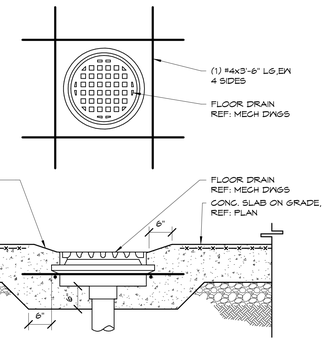
CONCRETE COLUMN NOTE:
DETAIL REPRESENT STEEL COLUMNS & BASE PLATES APPLY SAME TECHNIQUES & MINIMUM DISTANCES FOR CONCRETE COLUMN CONSTRUCTION.



TYPICAL SLAB ISOLATION JOINT

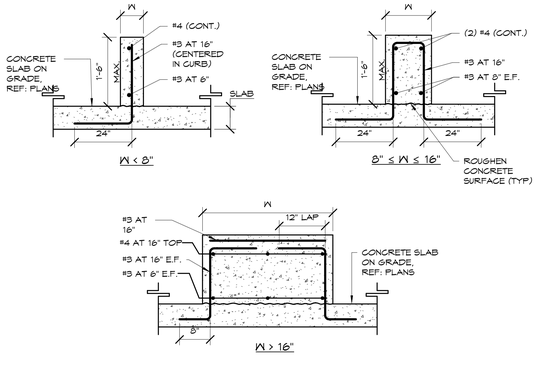


SLAB STEP DETAILS

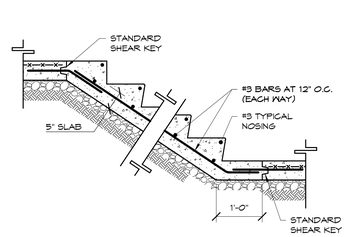


TYPICAL SLAB ON GRADE AT FLOOR DRAIN

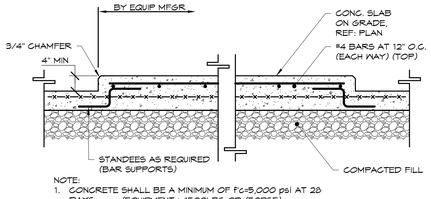
RAILING ANCHORAGE:
WHEN CURB IS SUPPORTING SAFETY RAILING OR SCREENING, CONTRACTOR SHALL COORDINATE ANCHORAGE DESIGN & INSTALLATION PRIOR TO CONSTRUCTION. SUBMIT ANCHORAGE DESIGN TO E.O.R. FOR REVIEW / APPROVAL.



TYPICAL CONCRETE CURB DETAILS

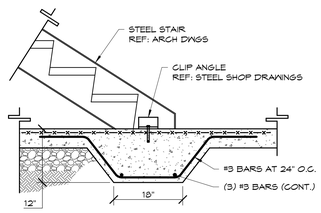


TYPICAL STAIR ON GRADE

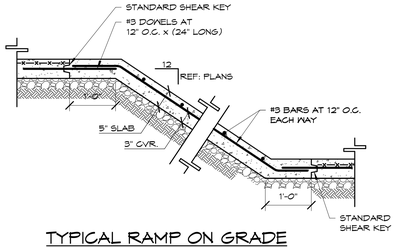


NOTE:
1. CONCRETE SHALL BE A MINIMUM OF f'c=3,000 psi AT 28 DAYS (EQUIPMENT > 1500LBS OR (150PSF))

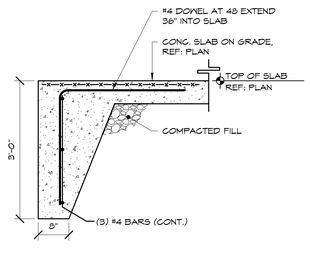
TYPICAL EQUIPMENT/ HOUSEKEEPING PAD DETAIL



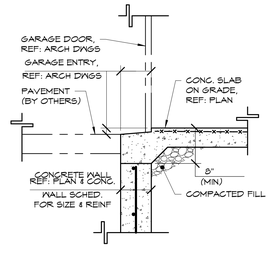
HAUNCHED SLAB ON GRADE AT STEEL STAIR



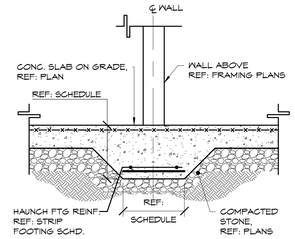
TYPICAL RAMP ON GRADE



TYPICAL TURNDOWN FOOTING DETAIL



TYPICAL GARAGE ENTRY SLAB DETAIL



NON-LOAD BEARING CMU PARTITION NOTE:
ALL NON-LOAD BEARING CMU WALLS SHALL RECEIVE A 18"(W)x12"(D) HAUNCHED SLAB WITH (2) #3 BARS CONTINUOUS AND #3 BARS AT 36" O.C. TRANSVERSE COORDINATE ALL WALL LOCATIONS WITH ARCHITECTURAL DRAWINGS.

TYPICAL HAUNCHED SLAB DETAIL

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
TYPICAL SLAB ON GRADE DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:43:10 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

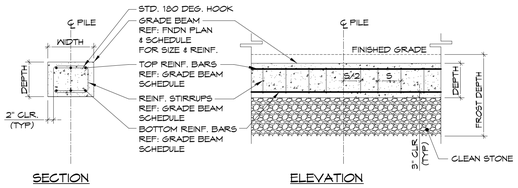
SCALE: As Noted
DATE:
PROJECT NO.: 2318_07

DRAWING NO.:
S502

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

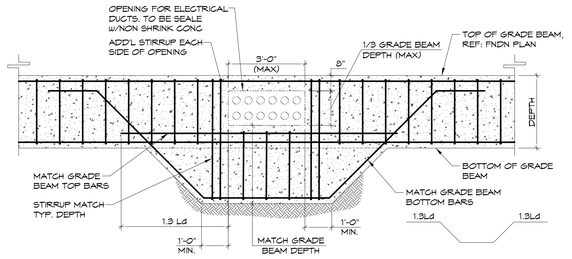
6/24/2024 8:16:26 AM



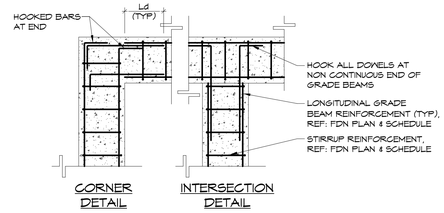
SECTION ELEVATION

EXTERIOR / PERIMETER GRADE BEAMS:
IF BOTTOM OF GRADE BEAM DOES NOT MEET MINIMUM FROST DEPTH, INSTALL 3/4" CLEAN STONE DOWN TO FROST DEPTH.

TYPICAL GRADE BEAM

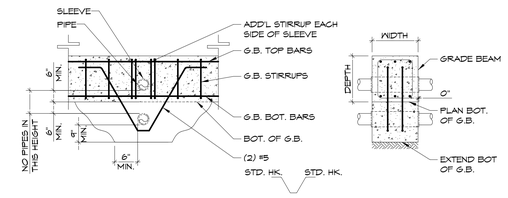


TYPICAL DETAIL OF DUCT BANK THROUGH GRADE BEAM



CORNER DETAIL INTERSECTION DETAIL

TYPICAL HORIZONTAL REINFORCEMENT DETAIL CAST-IN-PLACE GRADE BEAM



ELEVATION SECTION

ALL CONCRETE GRADE BEAM PENETRATIONS SHALL BE SHOWN AND INDICATED IN PILE FOUNDATION PLAN & CONCRETE GRADE BEAM SCHEDULE. ANY ADDITIONAL PENETRATION THROUGH THE GRADE BEAMS MUST BE SUBMITTED TO E.O.R. FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

TYPICAL PIPE SLEEVE DETAILS AT GRADE BEAMS

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD, SUITE 402
LAKEWOOD, NJ 08701
SHIMON GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHIMON GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

GRADE BEAM TYPICAL DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:43:32 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

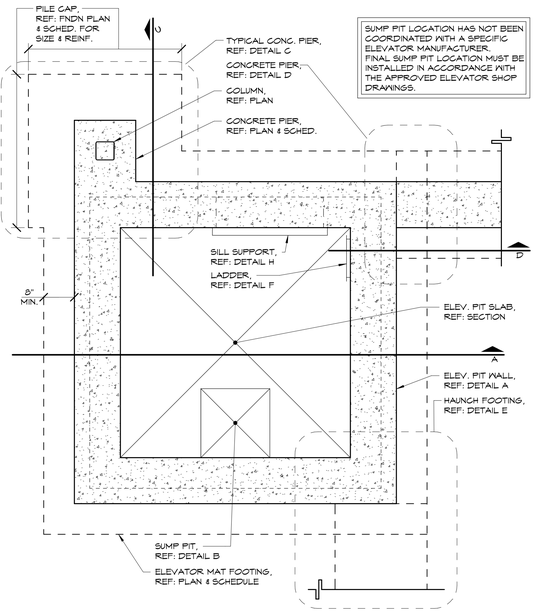
PROJECT NO. 2318_07

DRAWING NO. S504

BARCODE:

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

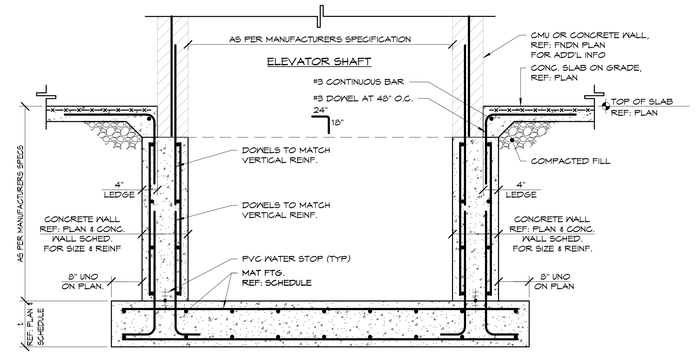
6/24/2024 8:16:26 AM



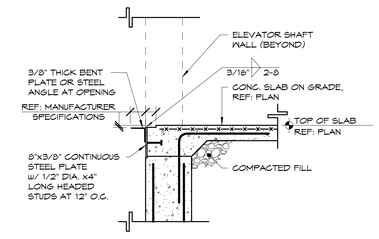
ELEVATOR PIT WALL (CMU ALTERNATE)
ELEVATOR PIT FOUNDATION WALL IS PERMITTED TO BE CONSTRUCTED UTILIZING 12" CMU BLOCK w/ #5 BARS AT 8" O.C. FILL ALL CELLS SOLID WITH 3,000PSI GROUT.

ELEVATOR PIT WATERPROOFING
REFERENCE ARCHITECTURAL DRAWINGS FOR WATERPROOFING REQUIREMENTS.

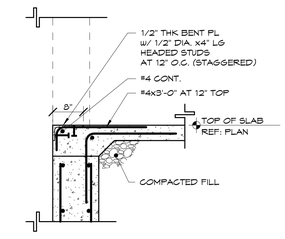
TYPICAL ELEVATOR PIT PARTIAL PLAN



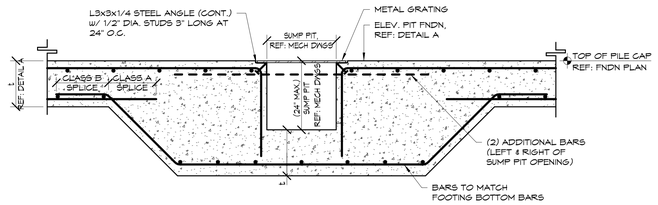
ELEVATOR PIT DETAIL
SCALE: 3/4"=1'-0"



TYPICAL ELEVATOR SILL AT OPENING DETAIL
SCALE: 3/4"=1'-0"

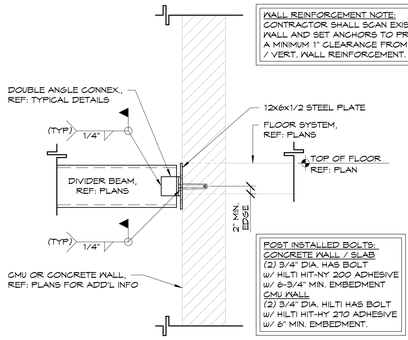


SILL AT FREIGHT ELEVATOR DOOR
SCALE: 3/4"=1'-0"



TYPICAL ELEVATOR SUMP PIT
SCALE: 3/4"=1'-0"

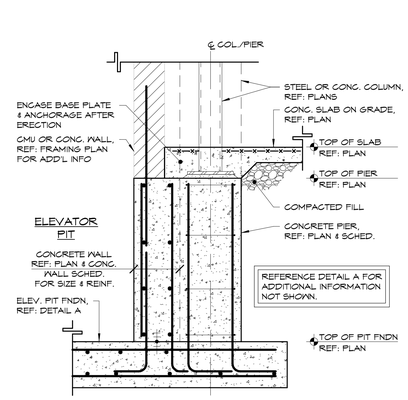
WALL REINFORCEMENT NOTE:
CONTRACTOR SHALL SCAN EXISTING WALL AND SET ANCHORS TO PROVIDE A MINIMUM 1" CLEARANCE FROM HORIZ. / VERT. WALL REINFORCEMENT.



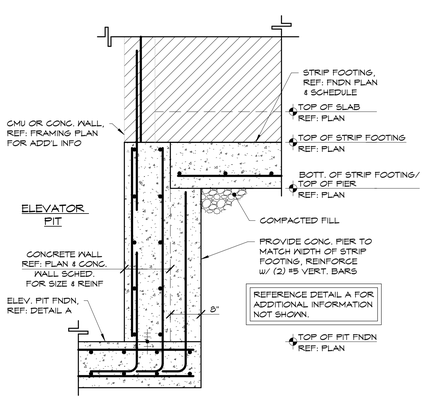
POST INSTALLED BOLTS:
CONCRETE WALL / SLAB
(2) 3/4" DIA. HAS BOLT w/ HILTI HIT-HY 200 ADHESIVE w/ 8-3/4" MIN. EMBEDMENT
CMU WALL
(2) 3/4" DIA. HILTI HAS BOLT w/ HILTI HIT-HY 270 ADHESIVE w/ 6" MIN. EMBEDMENT.

NOTES:
1. COORDINATE DIMENSION BETWEEN TOP OF FLOOR SYSTEM AND TOP OF DIVIDER BEAM WITH ELEVATOR MANUFACTURER.

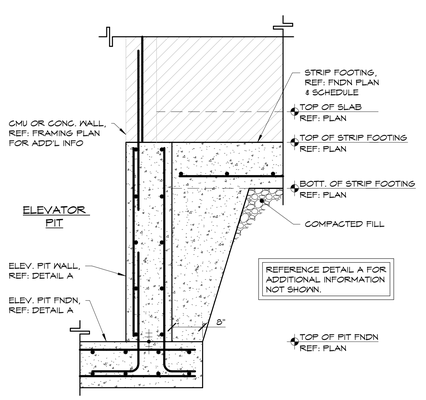
TYPICAL ELEVATOR DIVIDER BEAM (POST INSTALLED PLATE)
SCALE: 3/4"=1'-0"



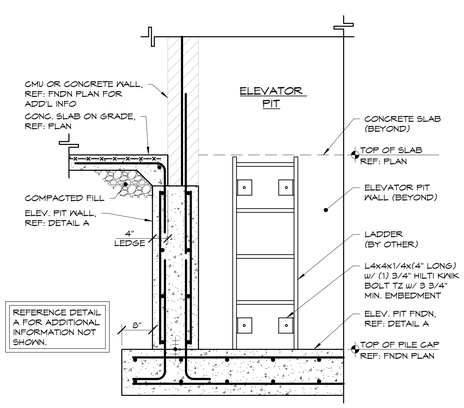
STEEL COLUMN & PIER DETAIL
SCALE: 3/4"=1'-0"



FOOTING SUPPORT AT PIER DETAIL
SCALE: 3/4"=1'-0"



HAUNCH FOOTING DETAIL
SCALE: 3/4"=1'-0"



ELEVATOR LADDER DETAIL
SCALE: 3/4"=1'-0"

RISE | FL architecture
ARCHITECT:
NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746
NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717
FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717
STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com
MEP CONSULTANT:
CIVIL CONSULTANT:
DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
TYPICAL ELEVATOR PIT DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:44:02 -04'00'

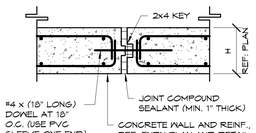
JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted
DATE:
PROJECT NO: 2318_07
DRAWING NO: S506

BARCODE

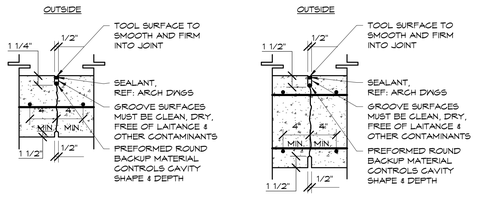
THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:29 AM



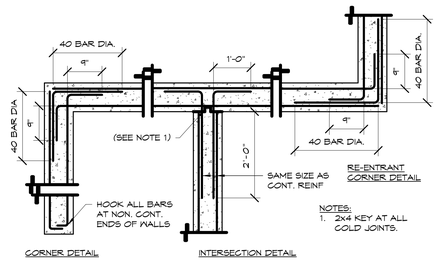
NOTES:
1. EXPANSION JOINTS SHALL BE PLACED AT A MAXIMUM OF 100FT O.C.
2. EXPANSION JOINTS SHALL BE PLACED A MAXIMUM OF 50 FT FROM BUILDING CORNERS
3. CONTRACTOR SHALL PROVIDE EOR/AOR WITH PLAN INDICATING PROPOSED EXPANION JOINT LOCATIONS FOR FINAL APPROVAL

CONCRETE WALL EXPANSION JOINT DETAIL



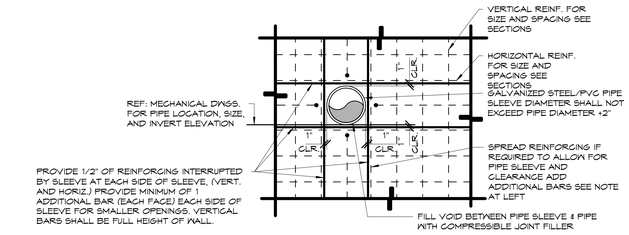
NOTES:
1. CONTROL JOINTS SHALL BE PROVIDED IN CONCRETE WALL CONSTRUCTION BY SAW-CUTTING JOINT IN WALL AS INDICATED.
2. CONTROL JOINTS SHALL BE SPACED EQUAL TO THE WALL HEIGHT WITH A MAXIMUM OF 25FT O.C. AND SHALL BE PROVIDED AT 15FT MAX. FROM WALL CORNERS.
3. COORDINATE JOINT LOCATION TO AVOID WALL VERTICAL REINFORCEMENT. MINIMUM 4" BETWEEN JOINT AND VERTICAL REINFORCEMENT.

CONCRETE WALL CONTROL JOINT DETAIL



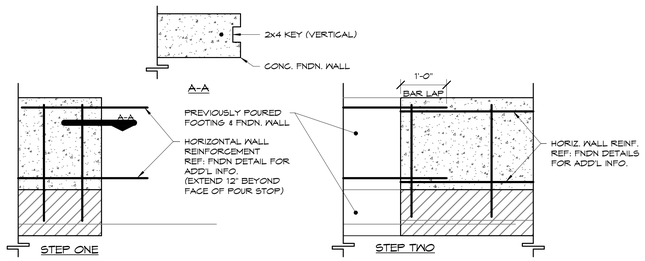
NOTES:
1. 2x4 KEY AT ALL COLD JOINTS.

TYPICAL HORIZONTAL REINFORCEMENT DETAIL FOR CAST-IN-PLACE CONCRETE WALL

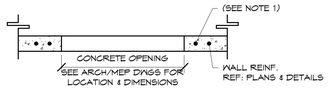


NOTES:
1. ALL PENETRATIONS THROUGH WALLS ARE SUBJECT TO APPROVAL BY STRUCTURAL ENGINEER.
2. MINIMUM CLEAR DISTANCE BETWEEN ADJACENT SLEEVES SHALL NOT BE LESS THAN 2'-0"
3. HOOK ALL INTERRUPTED CONTINUOUS BARS.

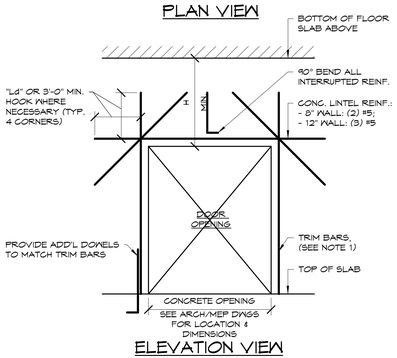
TYPICAL PIPE PENETRATION THROUGH CONCRETE FOUNDATION WALL



CONCRETE FOUNDATION WALL & STRIP FOOTING CONSTRUCTION JOINT DETAILS



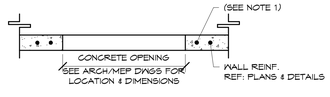
PLAN VIEW



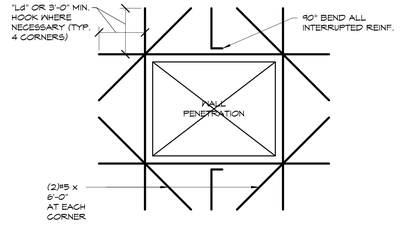
ELEVATION VIEW

NOTE:
1. ONE-HALF OF REINFORCING BAR AREA INTERRUPTED BY
2. OPENING SHALL BE PROVIDED EACH SIDE OF OPENING IN THE SAME FACE MINIMUM (2) #4 EACH SIDE
3. REFERENCE CONC. LINTEL REINFORCEMENT SCHEDULE FOR LINTEL REINFORCEMENT

TYPICAL CONCRETE LINTEL DETAIL



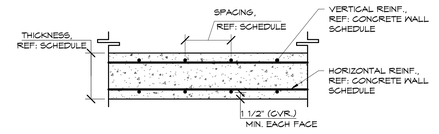
PLAN VIEW



ELEVATION VIEW

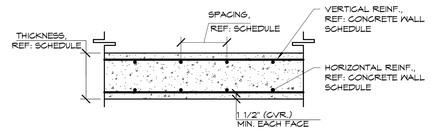
NOTES:
1. ONE-HALF OF REINFORCING BAR AREA INTERRUPTED BY OPENING SHALL BE PROVIDED EACH SIDE OF OPENING IN THE SAME FACE MINIMUM (2) #4 EACH SIDE
2. PROVIDE BEAM REINFORCEMENT OVER OPENINGS.

CONCRETE WALL OPENING REINFORCING DETAIL



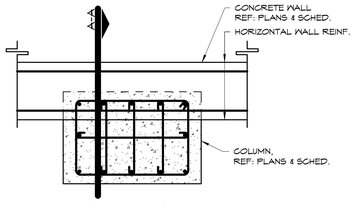
PLAN

TYPICAL CONCRETE WALL (VERTICAL SPANNING) (U.N.O.)

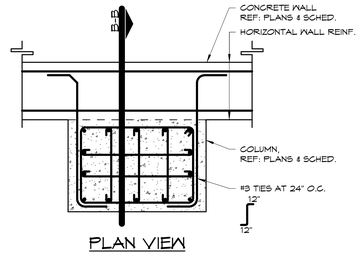


PLAN

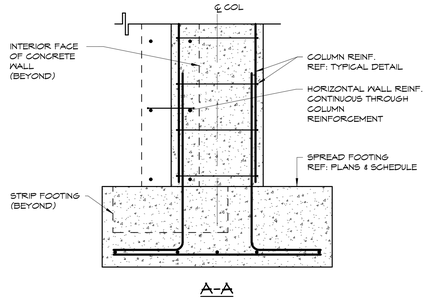
TYPICAL CONCRETE WALL (HORIZONTAL SPANNING)



PLAN VIEW

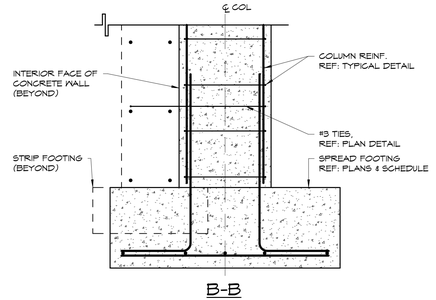


PLAN VIEW



A-A

CONCRETE WALL W/ INTEGRATED CONCRETE COLUMN/PIER



B-B

CONCRETE WALL W/ ADJACENT CONCRETE COLUMN/PIER

TYPICAL CONCRETE WALL CONSTRUCTION DETAIL

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
TYPICAL CONCRETE WALLS DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:44:23 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

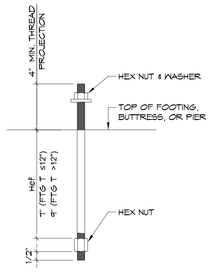
SCALE: As Noted
DATE:
PROJECT NO.: 2318_07

DRAWING NO.: S516

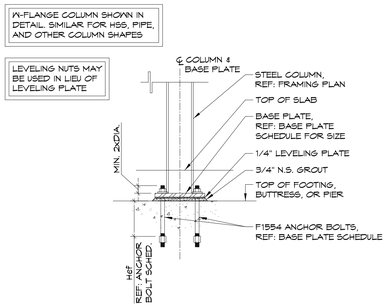
BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

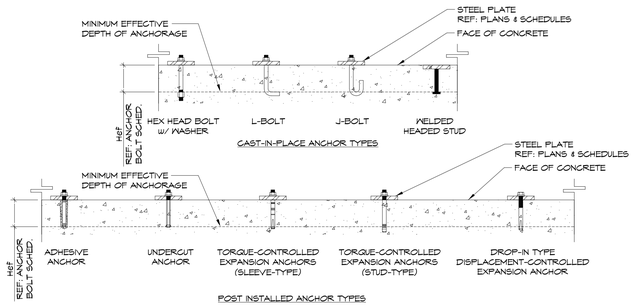
6/24/2024 8:16:30 AM



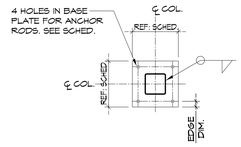
TYPICAL STEEL COLUMN ANCHOR BOLT DETAIL



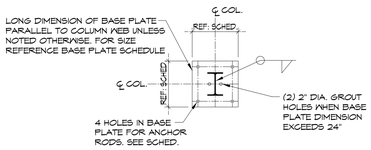
TYPICAL STEEL COLUMN BASE PLATE DETAIL



TYPICAL ANCHORAGE EMBEDMENT DEPTH



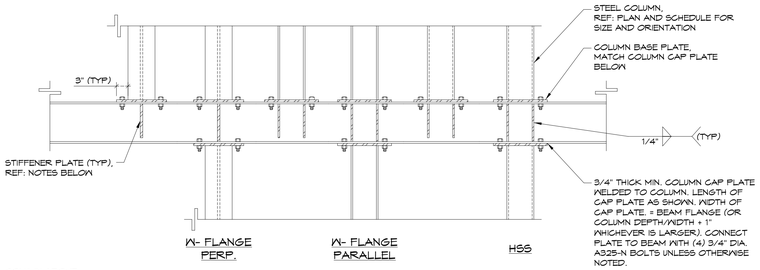
HSS COLUMN
PIPE COLUMN SIMILAR 4 ANCHOR BOLTS



"W" COLUMN
4 ANCHOR BOLTS

ANCHOR ROD HOLE SCHED.		
ANCHOR ROD DIA.	HOLE DIA.	EDGE DISTANCE
3/4"	1-5/16"	1-1/2"
7/8"	1-9/16"	1-1/2"
1"	1-7/8"	1-3/4"
1-1/4"	2-1/8"	2"
1-1/2"	2-3/8"	2-1/2"

TYPICAL STEEL COLUMN BASE PLATE DETAIL

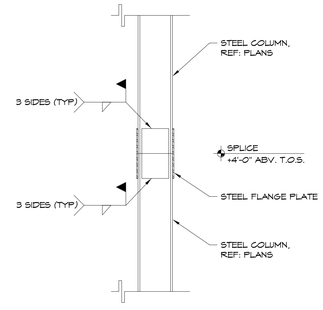


COLUMN ABOVE:
PROVIDE FULL DEPTH STIFFENER PLATE(S) (EACH SIDE) (3/8" THICK MIN.) REFERENCE DETAILS FOR CONDITION AND QUANTITY OF PLACE. ADJUST THICKNESS OF PLATE FOR GRADE OF MATERIAL

COLUMN BELOW:
PROVIDE FITTED STIFFENER PLATE(S) (EACH SIDE) (3/8" THICK MIN.) REFERENCE DETAILS FOR CONDITION AND QUANTITY OF PLACE. ADJUST THICKNESS OF PLATE FOR GRADE OF MATERIAL

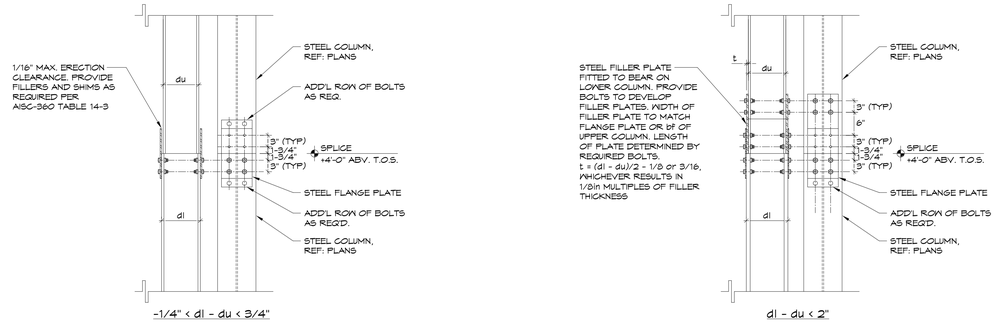
COLUMN ABOVE & BELOW:
PROVIDE FITTED STIFFENER PLATE(S) (EACH SIDE) (3/8" THICK MIN.) REFERENCE DETAILS FOR CONDITION AND QUANTITY OF PLACE. ADJUST THICKNESS OF PLATE FOR GRADE OF MATERIAL

BEAM OVER COLUMN & COLUMN OVER BEAM TYPICAL DETAIL



TYPICAL HSS STEEL GRAVITY COLUMN SPLICE DETAIL

NOTES:
1. PLATES SHALL BE SIZED TO PROVIDE MINIMUM WELD SHELF ON WORKABLE FLAT OF HSS.



TYPICAL W-FLANGE STEEL GRAVITY COLUMN SPLICE DETAIL

NOTES:
1. ALL BOLTS SHALL BE MIN. 3/4" DIA. A325-N BOLTS.
2. REFERENCE AISC CHAPTER 14 FOR ADD'L INFORMATION.

RISE | FL architecture
ARCHITECT:
NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746
NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMON GREENEBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717
FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENEBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717
STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com
MEP CONSULTANT:
CIVIL CONSULTANT:
DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
STEEL COLUMN AND BASE PLATE DETAILS

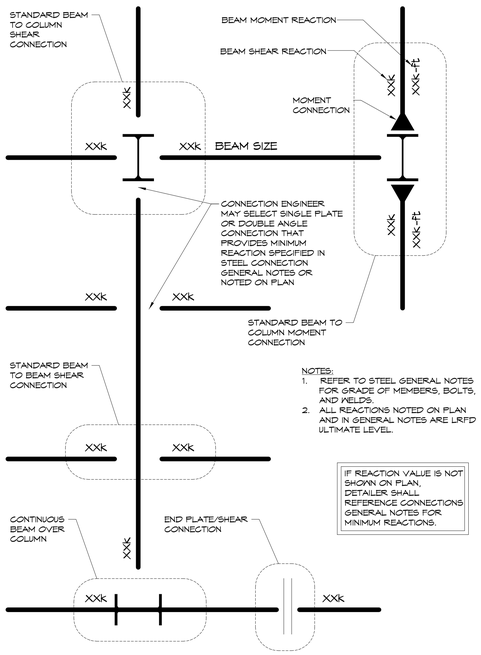
EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:44:43 -04'00'
JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted
DATE:
PROJECT NO: 2318_07
DRAWING NO: S521
BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

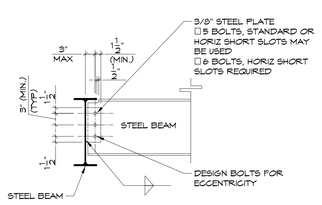
6/24/2024 8:16:31 AM



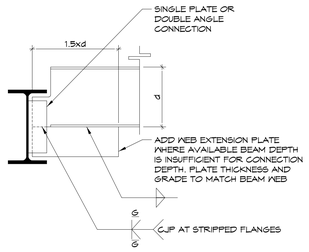
NOTES:
1. REFER TO STEEL GENERAL NOTES FOR GRADE OF MEMBERS, BOLTS, AND WELDS.
2. ALL REACTIONS NOTED ON PLAN AND IN GENERAL NOTES ARE LRFD ULTIMATE LEVEL.

IF REACTION VALUE IS NOT SHOWN ON PLAN, DETAILER SHALL REFERENCE CONNECTIONS GENERAL NOTES FOR MINIMUM REACTIONS.

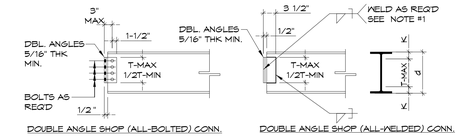
TYPICAL BEAM CONNECTIONS



TYPICAL SINGLE PLATE BEAM-TO-BEAM CONNECTION DETAIL

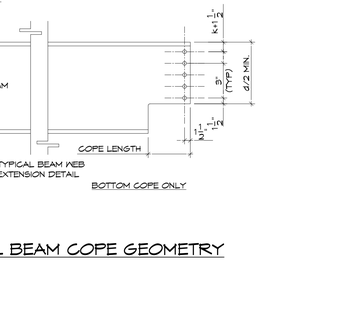


TYPICAL BEAM WEB EXTENSION DETAIL



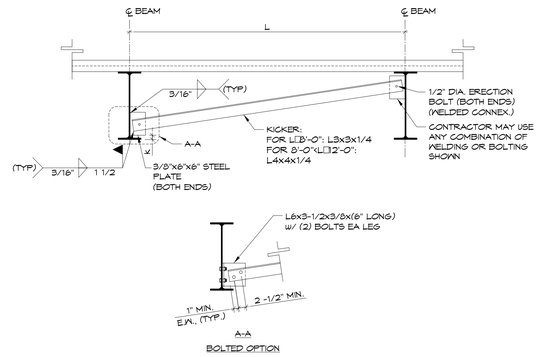
TYPICAL DOUBLE ANGLE BEAM-TO-BEAM CONNECTION DETAIL

NOTES:
1. LENGTH OF RETURN WELD TO BEAM SUPPORT ON TOP SIDE OF ANGLE SHALL BE EQUAL TO 2x THE SIZE OF FILLET.

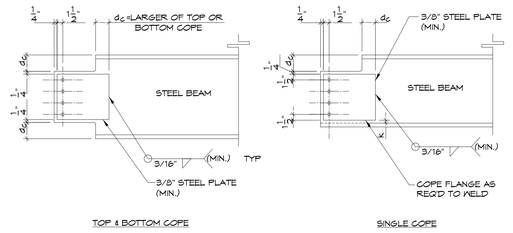


REFERENCE PLANS FOR MOMENT REACTIONS. IF REACTIONS ARE NOT PROVIDED, SUBMIT FORMAL REQUEST TO ENGINEER OF RECORD.

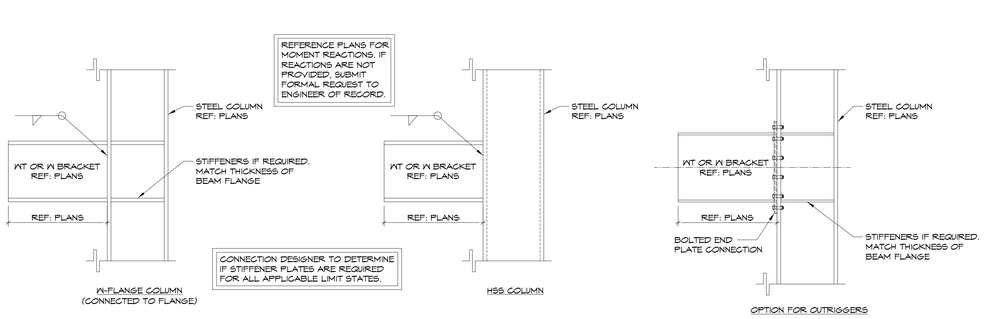
TYPICAL THRU-BEAM MOMENT CONNECTION DETAIL



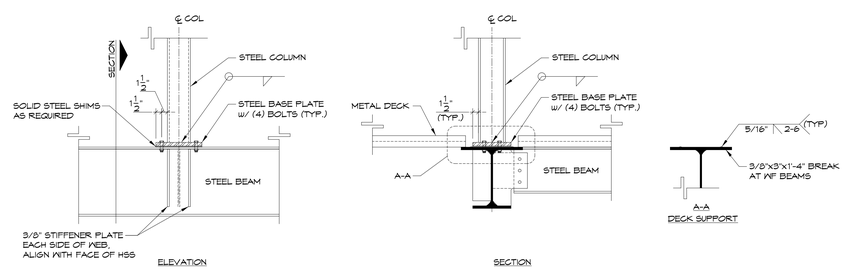
TYPICAL KICKER DETAIL



TYPICAL BEAM WEB REINFORCING



TYPICAL OUTRIGGER/BRACKET AT STEEL COLUMN DETAILS



TYPICAL COLUMN TRANSFER DETAIL

TYPICAL BEAM COPE GEOMETRY

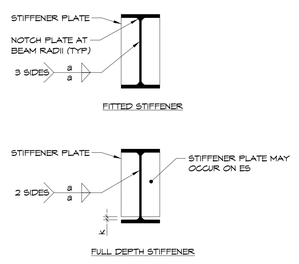


PLATE WELD SCHEDULE	
STIFFENER PLATE THICKNESS	WELD SIZE a (U.N.O.)
3/16"	3/16"
1/4"	3/16"
5/16"	1/4"
3/8"	1/4"
1/2"	5/16"
5/8"	7/16"
3/4"	1/2"
1"	5/8"
1 1/4"	7/8"

NOTES:
1. ACTUAL STIFFENER GEOMETRY VARIES. SEE DETAILS.
2. STIFFENER SURFACES THAT CONTACT BEAM FLANGES SHALL BE FINISHED FOR BEARING.

RISE | FL architecture

ARCHITECT:
NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
STEEL CONNECTIONS DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:45:08 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

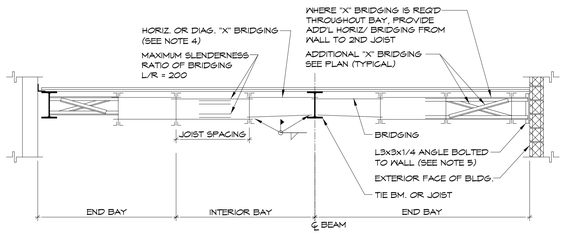
SCALE: As Noted
DATE:
PROJECT NO.: 2318_07

DRAWING NO.:
S522

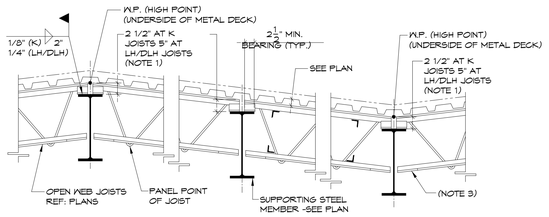
BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:32 AM



BRIDGING TERMINATION



SLOPED ROOF

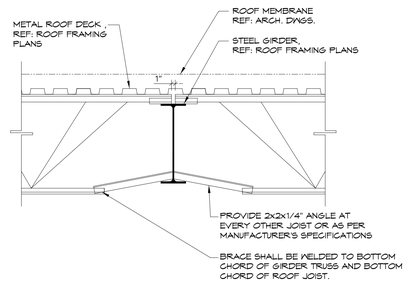
NOTES:
1. BEARING ENDS OF SLOPED JOISTS SHALL BE HORIZONTAL TO OBTAIN FULL CONTACT WITH SUPPORTING MEMBERS.
2. AT END BAYS, EXTEND BOTTOM CHORDS & CONNECT TO SPANDREL BEAMS AS SHOWN IN SECTIONS.
3. CEILING EXTENSIONS ARE REQUIRED.
4. BRIDGING SHALL BE INSTALLED IN ACCORDANCE WITH SJI STANDARDS OR AS NOTED ON PLAN (WHICHEVER IS MORE STRINGENT)
5. UNGROUTED CMU; BOLT STEEL ANGLE TO WALL WITH (2) 3/4" DIA. HILTI HAS BOLTS EPOXY SET WITH HILTI HIT HY-70 ADHESIVE WITH HILTI SCREEN TUBE.
   GROUTED CMU; BOLT STEEL ANGLE TO WALL w/ (2) 3/4" HILTI HAS BOLTS SET WITH HILTI HIT HY-70 ADHESIVE INTO WITH MIN. 6" EMBEDMENT

TYPICAL OPEN WEB JOIST DETAIL (FLAT & SLOPED ROOF)

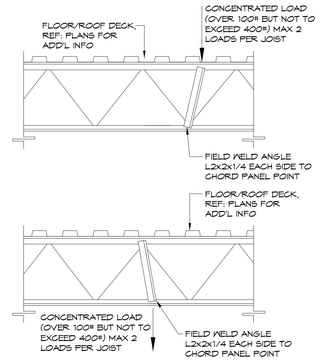


NOTES:
1. DECK SHEETS ARE TO BE FASTENED TO ALL SUPPORTS WITH NOT LESS THAN 5/8" DIA. PUDDLE WELDS.
2. PERIMETER ENDS FASTEN WITH WELDS AT 6" O.C..
3. PERIMETER SIDES FASTEN WITH WELDS AT 6" O.C..
4. INTERIOR SUPPORTS FAST WITH WELDS, SEE DIAGRAM FOR SPACING.
5. ENDLAPS FASTEN WITH WELDS AT 6" O.C..
6. SIDELAPS ARE TO BE FASTENED USING #10 TEK SCREWS AT MIDSPAN OR 3'-0" O.C. MAXIMUM WHEN SPANS EXCEEDS 5'-0".
7. AT DECK BUTT JOINTS, BOTH SHEETS ARE TO BE WELDED TO SUPPORTS.
8. ALL DECK ACCESSORIES OTHER THAN POUR STOPS AND FINISH STRIPS SHALL BE ATTACHED BY EITHER TACK WELDING OR #10 TEK SCREWS (BY OTHERS) AT 24" O.C. MAX ACCORDING TO SDI.

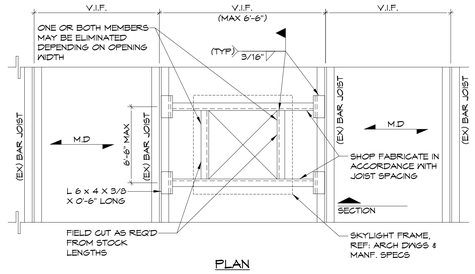
TYPICAL GUAGE METAL ROOF DECK ATTACHMENT SCHEDULE



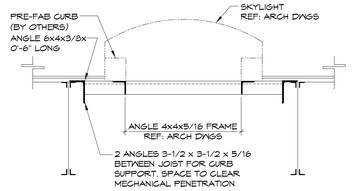
TYPICAL GIRDER BRACING DETAIL



TYPICAL JOIST REINF. AT POINT LOAD DETAIL

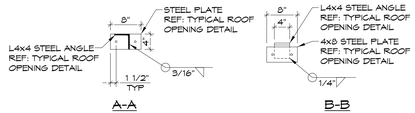


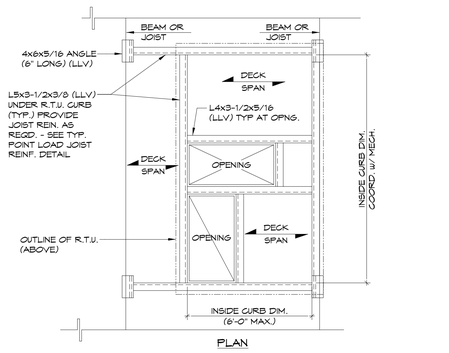
PLAN



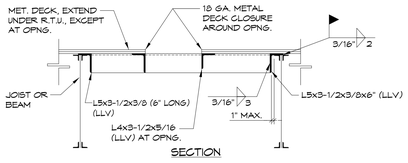
SECTION

TYPICAL SKYLIGHT DETAIL





PLAN

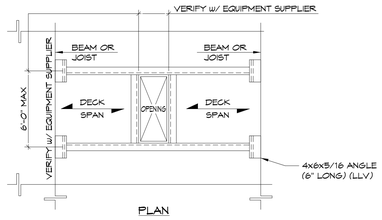


SECTION

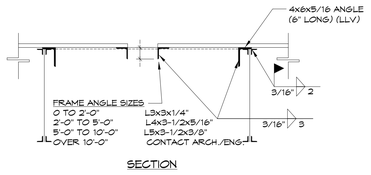
ROOF TOP UNIT / LARGE EQUIPMENT SUPPORT

NOTES:
1. COORDINATE SIZE & LOCATION OF CURBS & OPENINGS w/ MECH. CONTRACTOR.
2. ALIGN HEADER w/ TOP CHORD PANEL POINTS, OR PROVIDE JOIST REINF. - SEE TYP. POINT LOAD JOIST REINF. DETAIL

ROOF EQUIPMENT SUPPORT DETAIL



PLAN

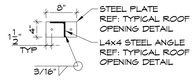


SECTION

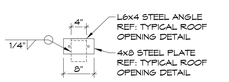
EXHAUST FAN / SMALL EQUIPMENT SUPPORT

NOTES:
1. COORDINATE SIZE & LOCATION OF CURBS & OPENINGS w/ MECH. CONTRACTOR.
2. ALIGN HEADER w/ TOP CHORD PANEL POINTS, OR PROVIDE JOIST REINF. - SEE TYP. POINT LOAD JOIST REINF. DETAIL

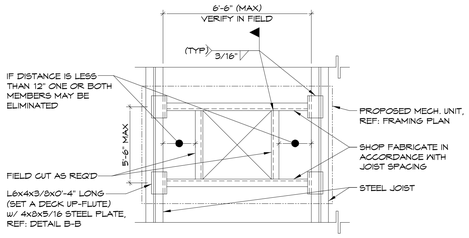
ROOF EQUIPMENT SUPPORT DETAIL

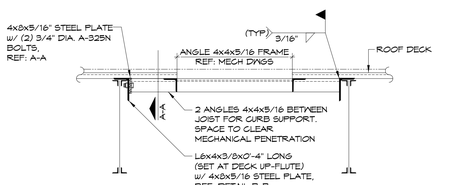


SECTION A-A



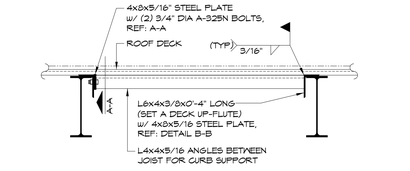
SECTION B-B



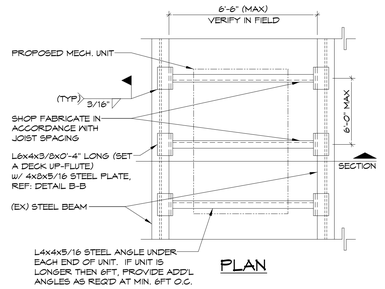


NOTE:
CONTRACTOR SHALL VERIFY OPENING SIZES AND PROVIDE FRAMES FOR OPENINGS LARGER THAN 12" BOTH WAYS.

TYPICAL ROOF PENETRATION DETAIL



SECTION



PLAN

TYPICAL ROOF REINF. DETAIL AT MECH. UNIT (WITHOUT DUCT PENETRATION)

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHIMON GREENBAUM, RA
SHIMON@RISEARCHITECTURE.COM
908-674-8711

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHIMON GREENBAUM, RA
SHIMON@RISEARCHITECTURE.COM
908-674-8711

STRUCTURAL CONSULTANT:

JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
STEEL BAR JOISTS TYPICAL DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:45:27 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

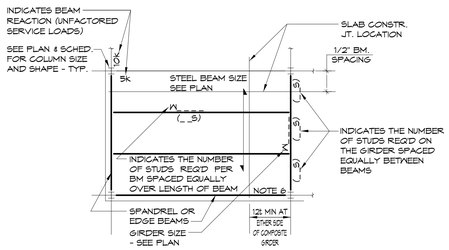
SCALE: As Noted
DATE:
PROJECT NO. 2318_07

DRAWING NO.: S524

BARCODE

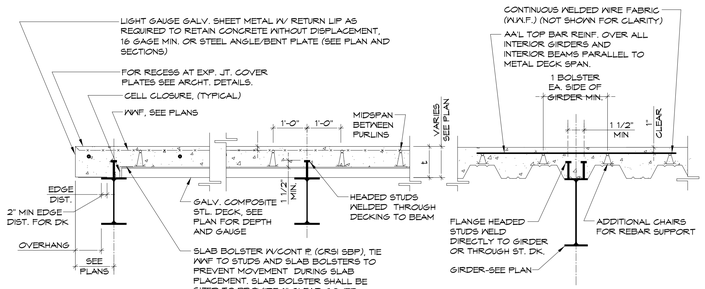
THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:32 AM



KEY PLAN

NOTES:
1. STEEL DECK & TOP OF FLANGES OF STEEL BEAMS SHALL BE FREE OF PAINT, ICE, WATER, DIRT, EXCESSIVE SCALE, RUST OR OTHER MATERIAL INJURIOUS TO WELDING.
2. STEEL DECK SHALL REST TIGHTLY AGAINST THE STEEL BEAM TOP FLANGE.
3. STUDS SHALL BE WELDED ONLY THROUGH A SINGLE THICKNESS OF METAL DECK.
4. FOR DECK SUPPORT AT COLUMN REFERENCE TYPICAL DETAIL.
5. CONTRACTOR SHALL PROVIDE LEVEL FLOORS TO MEET THE PARTITION REQUIRES. THE FLOOR LEVELESS MAY BE OBTAINED EITHER BY SCREENING CONCRETE, LEVEL IRRESPECTIVE OF BEAM & STEEL DECK DEFLECTION OR BY FLASH PATCHING AFTER ALL FLOOR SLABS ARE IN PLACE AND SHORING REMOVED.



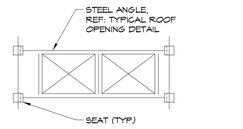
AT EXTERIOR OR EDGE BEAM

AT INTERIOR BEAM

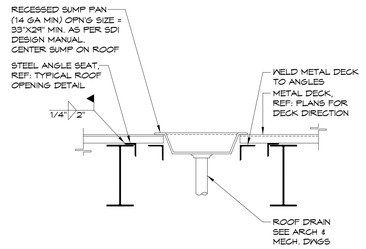
SECTION X-X

SECTION Y-Y
(FOR BMS PARALLEL TO SPAN OF STL DK.)

TYPICAL COMPOSITE BEAM DETAILS

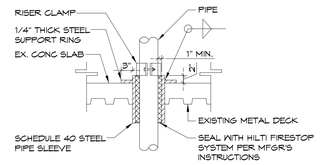


PLAN

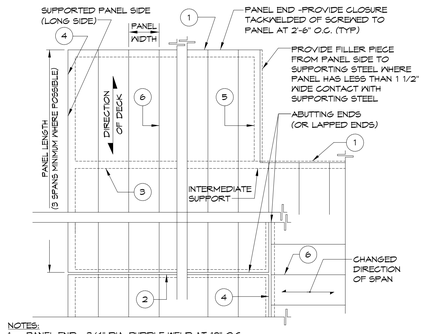


SECTION

TYPICAL SUMP PAN DETAIL

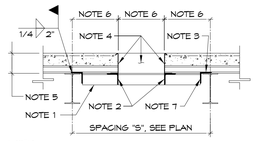


TYPICAL PIPE PENETRATION THRU SLAB ON METAL DECK DETAIL



NOTES:
1. PANEL END - 3/4" DIA. PUDDLE WELD AT 12" O.C. BUTTING ENDS - 3/4" DIA. PUDDLE WELDS AT 12" O.C. EACH PANEL.
2. LAPPED ENDS - 3/4" DIA. PUDDLE WELDS AT 12" O.C. AT CENTER OF LAPPED ENDS.
3. PANEL INTERMEDIATE SUPPORT - REFERENCE TYPICAL GAUGE METAL FLOOR DECK ATTACHMENT SCHEDULE
4. PANEL SIDE - REFERENCE TYPICAL GAUGE METAL FLOOR DECK ATTACHMENT SCHEDULE
5. PANEL SIDE WITH FILLER PIECE - REFERENCE TYPICAL GAUGE METAL FLOOR DECK ATTACHMENT SCHEDULE
6. PANEL SIDE LAP - REFERENCE TYPICAL GAUGE METAL FLOOR DECK ATTACHMENT SCHEDULE
7. DASHED LINES INDICATE PERMANENT STEEL SUPPORTING MEMBERS

TYPICAL METAL FLOOR DECK ERECTION DETAIL



NOTES:
1. FOR "S" LESS THAN 4'-0" USE L3x3x5/16; FOR "S" LESS THAN 6'-0" USE L4x4x5/16; FOR "S" MORE THAN 6'-0" USE W-BEAM, DEPTH OF W-BEAM >= (SPAN(IN.)/24) MIN; THICKNESS OF W-BEAM'S WEB = 1/4".
2. MATCH ANGLE SIZE FROM NOTE #1.
3. SEAT SAME SIZE AS ANGLE (NOTE #1).
4. GAUGE METAL POUR STOP.
5. SLAB THICKNESS, SEE PLAN.
6. FOR OPENING SIZE & LOCATIONS SEE ARCH. AND MECH. DRAWINGS.
7. SHOP WELDED FRAME.

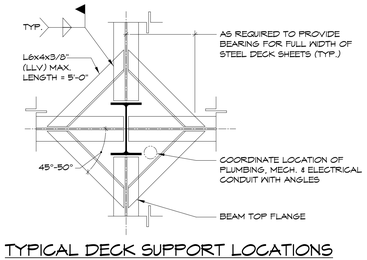
TYPICAL FRAMED FLOOR OPENING DETAIL

UNLESS NOTED OTHERWISE ON PLAN



NOTES:
1. DECK SHEETS ARE TO BE FASTENED TO ALL SUPPORTS WITH NOT LESS THAN 3/4" DIA. PUDDLE WELDS.
2. PERIMETER ENDS FASTEN WITH WELDS AT 12" O.C..
3. PERIMETER SIDES FASTEN WITH WELDS AT 12" O.C..
4. INTERIOR SUPPORTS FAST WITH WELDS, SEE DIAGRAM FOR SPACING.
5. ENDLAPS FASTEN WITH WELDS AT 12" O.C.
6. SIDELAPS ARE TO BE FASTENED USING BUTTON PUNCHES OR WELDS AT MIDSPAN OR 3'-0" O.C. MAXIMUM WHEN SPANS EXCEEDS 5'-0".
7. AT DECK BUTT JOINTS, BOTH SHEETS ARE TO BE WELDED TO SUPPORTS.
8. POUR STOP SHALL BE WELDED WITH 1" FILLET WELDS AT 12" O.C. MAX. ACCORDING TO SDI WITH 2" MIN. BEARING.
9. ALL DECK ACCESSORIES OTHER THAN POUR STOPS AND FINISH STRIPS SHALL BE ATTACHED BY EITHER TACK WELDING OR #10 TEK SCREWS (BY OTHERS) AT 24" O.C. MAX ACCORDING TO SDI.

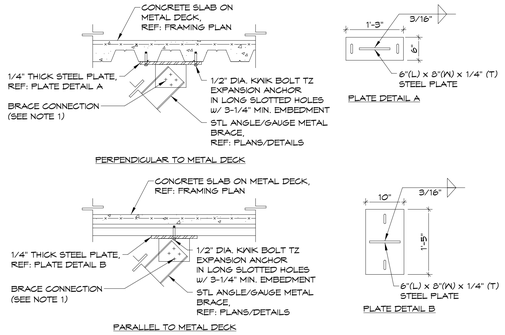
TYPICAL GAUGE METAL FLOOR DECK ATTACHMENT SCHEDULE



TYPICAL DECK SUPPORT LOCATIONS

NOTES:
1. PROVIDE METAL DECK SUPPORT:
   - AT BEAM TO BEAM CONNECTION.
   - WHERE PIPE SLEEVE OCCURS NEXT TO COLUMN.
   - WHERE COLUMN BASE OCCURS ON TOP OF STEEL BEAM.
   - AT ALL OTHER CONDITIONS WHERE DECK SUPPORT IS INTERRUPTED FOR A DISTANCE GREATER THAN 6".

TYPICAL METAL DECK SUPPORT AT COLUMN



NOTES:
1. CONNECTION FOR SHALL BE:
   - STEEL ANGLE
     - 1/8" ERECTION BOLT
     - 4" OF 1/4" FILLET WELD
   - GAUGE METAL STUD
     - (4) QUICKFLEX FASTENERS

TYPICAL BRACING CONNECTION TO SLAB ON METAL DECK

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
COMPOSITE METAL DECK TYPICAL DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino

Digitally signed by Jason Tarantino
Date: 2024.06.24 11:45:51 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

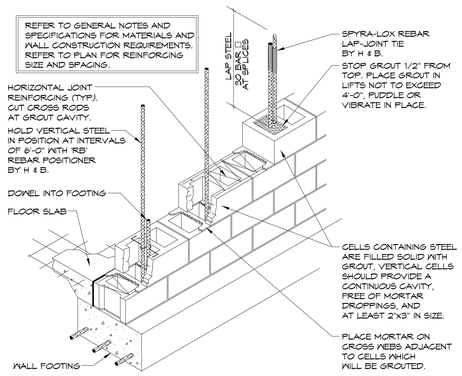
PROJECT NO: 2318_07

DRAWING NO:
S525

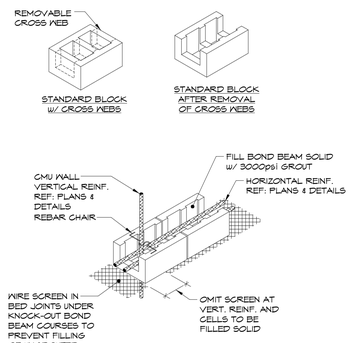
BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

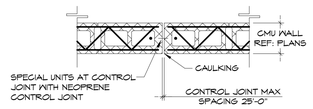
6/24/2024 8:16:33 AM



TYPICAL CONCRETE MASONRY WALL CONSTRUCTION

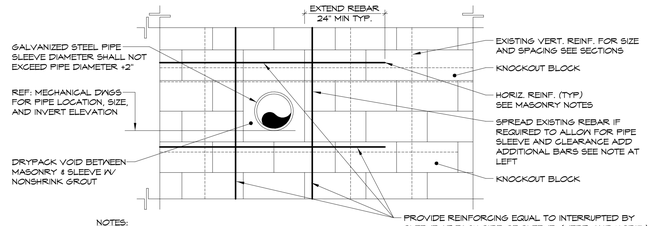


TYPICAL BOND BEAM CONSTRUCTION



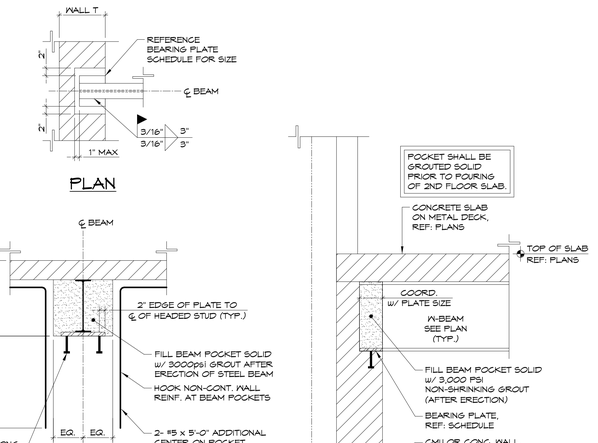
NOTES:
1. CONTROL JOINTS TO OCCUR FULL HEIGHT OF WALL
2. HORIZONTAL JOINT REINFORCING TO BE DISCONTINUOUS
3. BOND BEAM REINFORCING TO BE CONTINUOUS. WRAP BARS IN GREASE COATED KRAP OR PROVIDE JOINT STABILIZER ANCHORS.

TYPICAL MASONRY EXPANSION CONTROL JOINT DETAIL

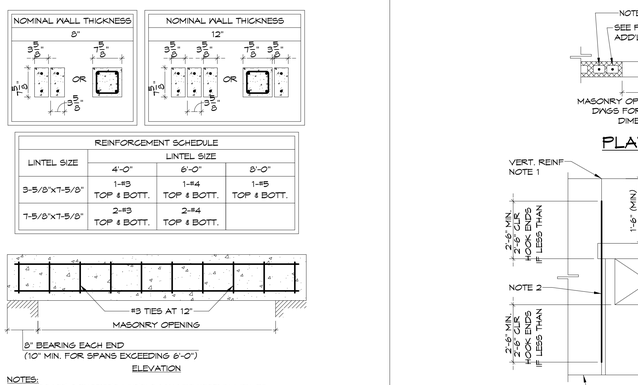


NOTES:
1. ALL PENETRATIONS THROUGH WALLS ARE SUBJECT TO APPROVAL BY STRUCTURAL ENGINEER MINIMUM.
2. CLEAR DISTANCE BETWEEN ADJACENT SLEEVES SHALL NOT BE LESS THAN 2'-0" HOOK ALL.
3. INTERRUPTED CONTINUOUS BARS.

TYPICAL PIPE PENETRATION THROUGH MASONRY FOUNDATION WALL

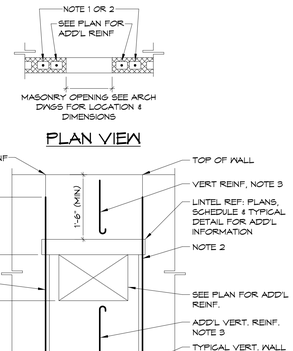


TYPICAL BEAM POCKET DETAIL



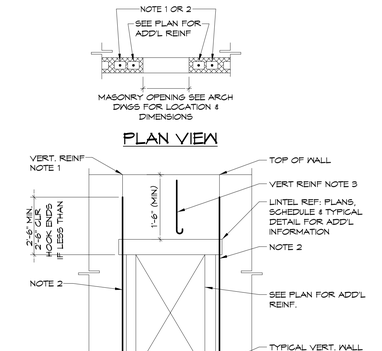
NOTES:
1. PRECAST CONCRETE LINTELS MAY BE USED IN LIEU OF REINFORCED CONCRETE MASONRY LINTELS.
2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE f'c=3000psi.
3. IF MASONRY OPENINGS IS NEXT TO A COLUMN, DO NOT USE PRECAST CONCRETE LINTEL. USE STRUCTURAL STEEL LINTEL AND CONNECT TO COLUMN.
4. FOR MASONRY OPENING SIZE AND LOCATION, SEE ARCHITECTURAL & STRUCTURAL DRAWINGS.
5. IF ANY UNUSUAL OPENING CONDITIONS EXISTS, CONSULT THE ENGINEER OF RECORD PRIOR TO FABRICATING LINTEL. THE ENGINEER OF RECORD RESERVES THE RIGHT TO MODIFY THIS DETAIL, IF WARRANTED BY SUCH A SITUATION.

TYPICAL PRECAST CONCRETE LINTEL SCHEDULE AND DETAIL



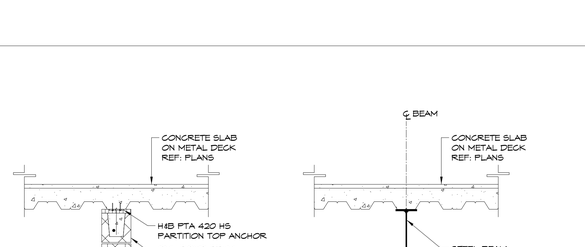
NOTES:
1. REINFORCED WALL: ALL INTERRUPTED VERT. BARS SHALL BE COMPENSATED FOR BY ADDITIONAL BARS ON EACH SIDE OF OPENING FULL HEIGHT OF WALL TOTAL AREA OF ADD'L BARS SHALL EQUAL AREA OF INTERRUPTED BARS. PROVIDE MIN (2) #5 EACH SIDE OF OPENING.
2. UNREINFORCED WALL: PROVIDE MIN (2) #5 EACH SIDE OF OPENING.
3. REINFORCED WALL: SIZE & SPACING OF REINF INDICATED ON PLAN. PROVIDE HOODED ENDS ON ALL DISCONTINUOUS BARS.

MASONRY WALL OPENING REINFORCING DETAIL

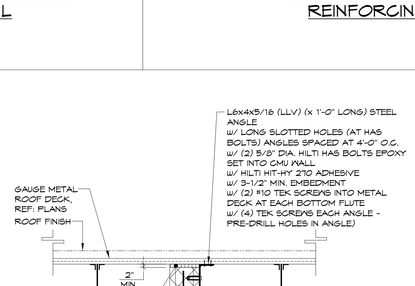


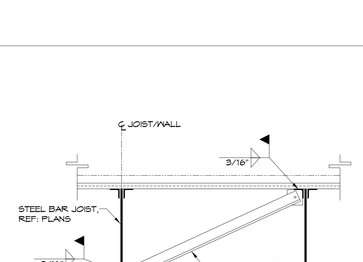
NOTES:
1. REINFORCED WALL: ALL INTERRUPTED VERT. BARS SHALL BE COMPENSATED FOR BY ADDITIONAL BARS ON EACH SIDE OF OPENING FULL HEIGHT OF WALL TOTAL AREA OF ADD'L BARS SHALL EQUAL AREA OF INTERRUPTED BARS. PROVIDE MIN (2) #5 EACH SIDE OF OPENING.
2. UNREINFORCED WALL: PROVIDE MIN (2) #5 EACH SIDE OF OPENING.
3. REINFORCED WALL: SIZE & SPACING OF REINF INDICATED ON PLAN. PROVIDE HOODED ENDS ON ALL DISCONTINUOUS BARS.

MASONRY DOOR OPENING REINFORCING DETAIL



TYPICAL MASONRY WALL BRACING DETAILS





TYPICAL MASONRY WALL BRACING DETAILS

RISE | FL architecture

ARCHITECT:
NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746
NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717
FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717
STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com
MEP CONSULTANT:
CIVIL CONSULTANT:
DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
MASONRY WALL CONSTRUCTION DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:46:12 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

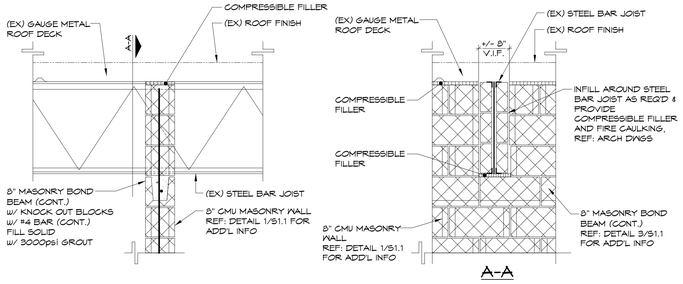
SCALE: As Noted
DATE:
PROJECT NO: 2318_07

DRAWING NO:
S531

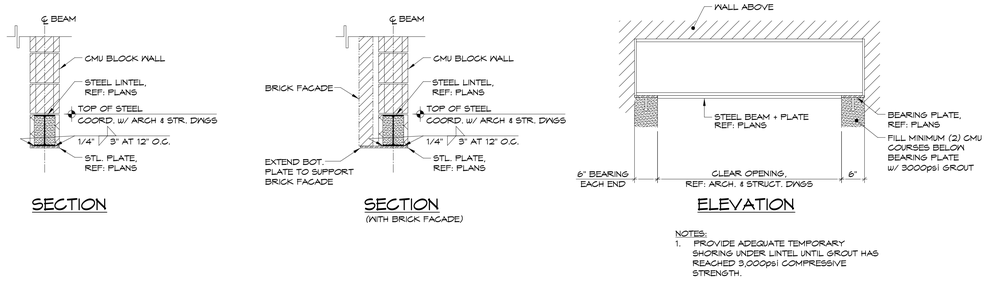
BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:33 AM

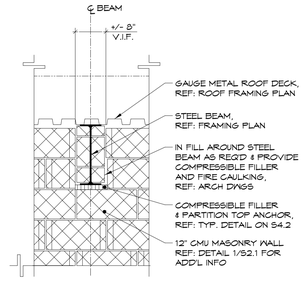


JOIST PENETRATION THRU NON-LOAD BEARING WALL



NOTES:
1. PROVIDE ADEQUATE TEMPORARY SHORING UNDER LINTEL UNTIL GROUT HAS REACHED 3,000psi COMPRESSIVE STRENGTH.

TYPICAL STEEL LINTEL DETAIL



BEAM PENETRATION THRU NON-LOAD BEARING WALL

TYPICAL MASONRY WALL INFILL DETAIL

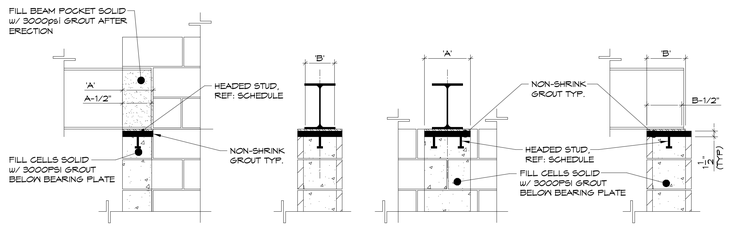
BEARING PLATE SCHEDULE

MK	LENGTH (A)	WIDTH (B)	THICKNESS	HEADED STUDS
BR1	8"	8"	1/2"	(1) 3/4" DIA. x 5" LONG

NOTES:
1. MASONRY WALL BELOW BEARING PLATES SHALL BE FILLED SOLID WITH 3000PSI GROUT.
2. ALL BEAM POCKETS SHALL BE FILLED SOLID WITH 3000PSI GROUT AFTER ERECTION OF STEEL.



STUD ANCHORS



TYPICAL STEEL BEARING PLATE DETAIL

RISE | FL architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, RA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 400
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMOM GREENBAUM, RA
SHMOM@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:
JT ENGINEERING BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:
Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:
MASONRY WALL LINTELS TYPICAL DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:46:34 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

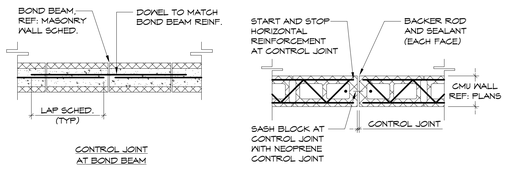
SCALE: As Noted
DATE:
PROJECT NO: 2318_07

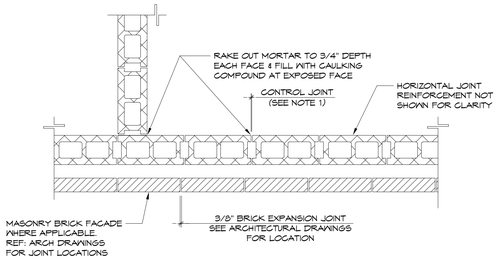
DRAWING NO:
S532

BARCODE

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:34 AM

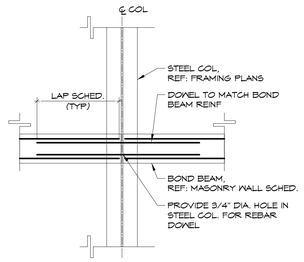




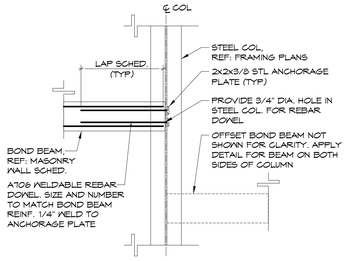
TYPICAL MASONRY CONTROL/EXPANSION JOINT DETAIL

NOTES:

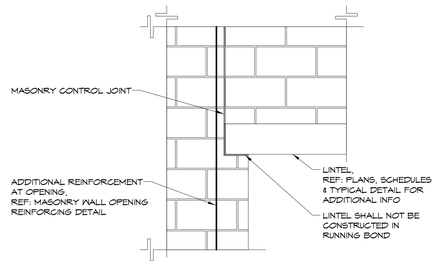
1. PROVIDE EXPANSION JOINT IN ACCORDANCE WITH PLANS. OTHERWISE THE FOLLOWING CRITERIA SHALL BE MET:
   - A. AT 1.5 X WALL HEIGHT (25'-0" O.C. MAX)
   - B. AT CHANGES IN WALL HEIGHTS.
   - C. INTERSECTIONS OF WALLS WITH COLUMNS, PIERS AND PILASTERS.
   - D. AT CORNERS.
   - E. NO CLOSER THAN 2'-0" TO EDGE OF ANY OPENING IN WALL
   - F. WALL THICKNESS CHANGES
   - G. SLAB CONTROL JOINTS
2. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF ALL CONTROL JOINTS IN BRICK FACADE.



BOND BEAM ALIGNED ON BOTH SIDES OF THE COLUMN

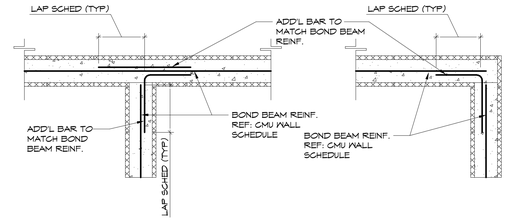


BOND BEAM NOT ALIGNED ON BOTH SIDES OF THE COLUMN



THIS DETAIL SHALL ONLY BE USED WHEN A CONTROL JOINT IS CALLED OUT AND LOCATED NEXT TO AN OPENING ON THE CONTROL JOINT PLAN.

MASONRY CONTROL JOINT AT OPENING DETAIL



BOND BEAM INTERSECTION DETAIL

RISE | FL
architecture

ARCHITECT:

NYC OFFICE
241 37TH STREET, SUITE 440
BROOKLYN, NEW YORK 11232
JEFF AKERMAN, PA
JEFF@RISEARCHITECTURE.COM
516-249-3746

NJ OFFICE
36 AIRPORT RD SUITE 402
LAKEWOOD, NJ 08701
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

FL OFFICE
241 37TH STREET, SUITE 440
CORAL SPRINGS, FL 33071
SHMON GREENBAUM, RA
SHMON@RISEARCHITECTURE.COM
908-674-8717

STRUCTURAL CONSULTANT:

JT ENGINEERING
BUILDING SOLUTIONS
1321 BRUNSWICK AVE,
Lawrence, NJ 08648
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

MEP CONSULTANT:

CIVIL CONSULTANT:

DESIGNER CONSULTANT:

REVISIONS:

NO.	DATE	DESCRIPTION
1	06/05/24	PERMIT

CLIENT:

PROJECT NAME / LOCATION:

Okeechobee (Grocery Store)
FL-78 & Access Road
Okeechobee, Florida, 34978

DRAWING TITLE:

MASONRY WALL TYPICAL DETAILS

EXAMINER'S SIGNATURE:

PROFESSIONAL SEAL:

Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2024.06.24 11:47:04 -04'00'

JASON C. TARANTINO
FL LIC. NO.
PE 94381

SCALE: As Noted

DATE:

PROJECT NO. 2318_07

DRAWING NO. S533

BARCODE:

THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND REPORT ALL ERRORS AND OMISSIONS TO THE ARCHITECTS. DO NOT SCALE DRAWING. THE DRAWING SHALL NOT BE USED UNTIL SIGNED BY CONSULTANTS.

6/24/2024 8:16:34 AM