FOOTING SCHEDULE 2,500 P.S.F. BEARING CAPACITY

MK	SIZE LENGTH	SIZE WIDTH	DEPTH (INCHES)	BOTT. REINF. EA-EACH WAY L-LONG S-SHORT	REMARKS
F3.0	3'-0"	3'-0"	12"	3 - #5	
F4.0	4'-0"	4'-0"	12"	4 - #5	
F4.5	4'-6"	4'-6"	12"	5 - #5	
F5.0x10.0	10'-0"	5'-0"	24"	4 - #7 (L) 9 - #7 (S)	
F5.5	5'-6"	5'-6"	12"	5 - #5	
F7.0x14.0	14'-0"	7'-0"	18"	6 - #7 (L) 15 - #7 (S)	
F8.0x16.0	16'-0"	8'-0"	18"	7 - #7 (L) 17 - #7 (S)	
F9.0x18.0	18'-0"	9'-0"	24"	8 - #8 (L) 19 - #7 (S)	
MF1.0	19'-2"	28'-0"	12"	#5 AT 12" O.C.	

NOTES:
1. SPREAD FOOTING SHALL BE NORMAL WEIGHT CONCRETE (f'c = 4,000 psi AT 28 DAYS).
2. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL CONFORM WITH THE "MINIMUM CONCRETE COVER FOR REINFORCING BAR" TABLE PROVIDED. (SEE ACI 318-14 SECTION 7.7 FOR CONDITIONS NOT NOTED)

SLAB ON GRADE CONSTRUCTION SCHEDULE

MARK	AREA	f'c (psi)	THICKNESS	REINFORCEMENT
①	INT. SOG	4,000	5"	WWF 6x6-W1.4xW1.4 (SEE NOTE 5)

NOTES:
1. ① INDICATES CONCRETE SLAB ON GRADE PLACED OVER A VAPOR BARRIER ON 6" OF CRUSHED STONE
2. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL CONFORM WITH THE "MINIMUM CONCRETE COVER FOR REINFORCING BAR" TABLE PROVIDED. (SEE ACI 318 SECTION 7.7 FOR CONDITIONS NOT NOTED).
3. ANY EXTERIOR STAIRS, RAMPS, OR ELEVATED PLATFORMS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 5000PSI CONCRETE.
4. CONTROL JOINTS IN SLAB ON GRADE SHALL BE LOCATED AND SPACED IN ACCORDANCE WITH TYPICAL CONTROL JOINT DETAIL ON S502.
5. CONTRACTOR MAY SUBSTITUTE 1.5 POUNDS OF FIBROUS REINFORCEMENT PER CUBIC YARD OF CONCRETE IN LIEU OF WWF.
6. REFERENCE TYPICAL CONCRETE SLAB DETAILS ON S502 FOR ADDITIONAL INFORMATION.

LOAD BEARING WALL LINTEL SCHEDULE

MK	TYPE	SIZE (W)x(H) (IN)	REINFORCEMENT	REMARKS
ML1	MASONRY	8x8	(2) #5 BARS	N/A
ML2	MASONRY	8x16	(2) #5 BARS	N/A
ML3	MASONRY	12x24	(2) #5 BARS (TOP & BOT.)	#5 ADD'L LONG BAR EACH FACE

NOTES:
1. REFERENCE S531 FOR ALL APPLICABLE TYPICAL DETAILS.

MASONRY WALL SCHEDULE

MK	THICKNESS	f'm	VERTICAL REINF.	HORIZ. REINF.	BOUNDARY REINF.	BOND BEAM SPACING (SEE NOTE 9)
MW8.1 (1,2)	8"	2,000psi	#5 BARS AT 24" O.C.	(SEE NOTE 4)	(1) #5 BARS	10'-0" O.C. (MAX.)
MW8.2 (1,2)	8"	2,000psi	#5 BARS AT 24" O.C.	(SEE NOTE 4)	(3) #5 BARS	10'-0" O.C. (MAX.)
MW8.3 (1,2)	8"	2,000psi	#7 BARS AT 16" O.C.	(SEE NOTE 4)	(1) #7 BARS	10'-0" O.C. (MAX.)
MW12.1 (1,2)	12"	2,000psi	#5 BARS AT 16" O.C.	(SEE NOTE 4)	(2) #5 BARS	10'-0" O.C. (MAX.)

NOTES:
1. MASONRY WALLS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF f'm = 2000psi U.N.O.. FILL ALL REINFORCED CELLS SOLID WITH 3000psi GROUT.
2. "MSW" DENOTES MASONRY SHEAR WALL. FILL ALL CELLS SOLID WITH 3,000psi GROUT. REFERENCE TYPICAL SHEAR WALL DETAIL ON S101A FOR ADDITIONAL INFORMATION.
3. WALL THICKNESS AND REINFORCEMENT SHALL APPLY TO ALL LEVELS OF WALL ABOVE POINT OF DESIGNATION OR UNTIL NEXT WALL DESIGNATION.
4. PROVIDE HORIZ. REINFORCEMENT (9 ga. GALVANIZED TRUSS TYPE) AT 16" O.C.
5. VERTICAL REINFORCEMENT BARS SHALL BE PLACED AT EACH WALL END AND CORNER THEN SPACED PER INDICATED SPACING.
6. PROVIDE 16" DEEP BOND BEAMS AT EACH FLOOR LEVEL w/ (2) #5 BARS CONTINUOUS. PROVIDE 8" DEEP BOND BEAMS BETWEEN FLOORS WHERE WALL IS GREATER THAN 10ft w/ (1) # 5 BAR CONTINUOUS. ALL BOND BEAMS SHALL HAVE KNOCK OUT BLOCKS AND SHALL BE FILLED SOLID WITH 3000PSI GROUT, REFERENCE TYPICAL DETAIL ON S531 FOR ADDITIONAL INFORMATION.
7. CONTRACTOR SHALL MAINTAIN RUNNING BOND THROUGHOUT WALLS AND CORNERS.
8. REFERENCE FRAMING PLANS FOR LINTELS, STEEL BEAMS, BEARING PLATES, AND LEDGER CONNECTION LOCATIONS.
9. REFERENCE TYPICAL CONCRETE MASONRY WALL CONSTRUCTION DETAILS ON S531 FOR ADDITIONAL INFORMATION.
10. REFERENCE TYPICAL MASONRY SHEAR WALL DETAILS FOR ADDITIONAL INFORMATION.

STRIP FOOTING SCHEDULE 2,500 P.S.F. BEARING CAPACITY

MK	SIZE WIDTH (INCHES)	SIZE DEPTH (INCHES)	REINFORCEMENT LONGITUDINAL	REINFORCEMENT TRANSVERSE
SF2.0	24"	12"	(3) #5 BARS	#5 BARS 36" O.C.

NOTES:
1. STRIP FOOTING SHALL BE NORMAL WEIGHT CONCRETE (f'c = 4,000 psi AT 28 DAYS).
2. MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL CONFORM WITH THE "MINIMUM CONCRETE COVER FOR REINFORCING BAR" TABLE PROVIDED. (SEE ACI 318-14 SECTION 7.7 FOR CONDITIONS NOT NOTED)

CONCRETE WALL SCHEDULE

MK	THICKNESS	NO OF CURTAINS	VERTICAL REINF.	HORIZONTAL REINF.	REMARKS
CW10.1	10"	1	#5 BARS AT 18" O.C.	#5 BARS AT 18" O.C.	
CW12.1	1'-0"	2	#5 BARS AT 18" O.C.	#5 BARS AT 18" O.C.	
CW18.1	1'-6"	2	#5 BARS AT 12" O.C.	#5 BARS AT 12" O.C.	

NOTES:
1. ALL CONCRETE WALLS SHALL BE 4000PSI CONCRETE.
2. WALL THICKNESS AND REINFORCEMENT SHALL APPLY TO ALL LEVEL OF WALL ABOVE POINT OF DESIGNATION OR UNTIL NEXT WALL DESIGNATION.
3. REFERENCE TYPICAL CONCRETE WALL CONSTRUCTION DETAILS ON S516 FOR ADDITIONAL INFORMATION.
4. (C) - CENTERED / (OF) - OUTSIDE FACE / (IF) - INSIDE FACE.

MASONRY REINFORCEMENT LAP SPLICE SCHEDULE (f'm=2,000psi)

BAR SIZE	6in WALL	8in WALL	10in WALL	12in WALL
#3	12"	12"	12"	12"
#4	18"	13"	12"	12"
#5	NS	20"	16"	13"
#6	NS	38"	29"	24"
#7	NP	52"	40"	33"
#8	NP	72"	61"	50"
#9	NP	NP	79"	64"

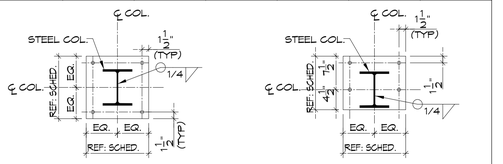
NOTES:
1. LAPS ARE APPLICABLE TO ONLY REBAR CENTERED IN THE WIDTH OF CMU WALL.
2. TABLE IS BASED ON FY = 60,000 PSI FOR GRADE 60 STEEL. APPLIES TO BOTH ALLOWABLE STRESS DESIGN AND STRENGTH DESIGN.
3. NP INDICATES REBAR NOT PERMITTED FOR USE IN CMU WALL WIDTH.
4. NS INDICATES REBAR THAT CANNOT BE SPLICED IN CMU WALL WIDTH.
5. BARS LARGER THAN NO. 9 TO BE SPLICED BY MECHANICAL CONNECTORS.
6. LENGTHS TABULATED MUST BE MULTIPLIED BY 1.5 FOR EPOXY COATED REBAR. DEVELOPMENT LENGTH NOT TO EXCEED 72"Db.

STEEL COLUMN SCHEDULE

MK	COLUMN SIZE
C1	W8X35

BASE PLATE SCHEDULE

MK	LENGTH	WIDTH	THICKNESS	ANCHOR BOLTS	EMBEDMENT	BASE PLATE TYPE
BP1	1'-2"	1'-2"	3/4"	(4) 3/4" DIA. F1554	(SEE NOTE 1)	TYPE 1
BP2	1'-0"	1'-2"	3/4"	(4) 3/4" DIA. F1554	(SEE NOTE 1)	TYPE 2



NOTES:
1. SEE TYPICAL COLUMN ANCHOR BOLT DETAIL FOR ANCHOR BOLT EMBEDMENT.

4,000 PSI NORMAL WEIGHT

BAR SIZE	db	LAP CLASS	TOP BARS CASE 1	TOP BARS CASE 2	OTHER BARS CASE 1	OTHER BARS CASE 2
#3	.375	EMBEDMENT (A)	19	28	15	22
		SPLICE (B)	24	36	19	28
#4	.50	EMBEDMENT (A)	25	37	19	29
		SPLICE (B)	32	48	25	37
#5	.625	EMBEDMENT (A)	31	47	24	36
		SPLICE (B)	40	60	31	47
#6	.75	EMBEDMENT (A)	37	56	29	43
		SPLICE (B)	48	72	37	56
#7	.875	EMBEDMENT (A)	54	81	42	63
		SPLICE (B)	70	106	54	81
#8	1.0	EMBEDMENT (A)	62	93	48	71
		SPLICE (B)	80	121	62	93

STRUCTURAL ELEMENT	CONCRETE COVER	CASE ACCORDING TO CENTER-TO-CENTER BAR SPACING ≤ 2db	> 2db < 3db	> 3db
BEAMS, COLUMNS, AND LAYER OF WALLS OR SLABS.	≤ db	2	2	2
	> db	2	1	1
ALL OTHERS	≤ db	2	2	2
	> db	2	2	1

NOTES:
1. TENSION DEVELOPMENT LENGTHS AND LAP SPLICE LENGTHS ARE CALCULATED PER ACI 318-14, SECTIONS 25.4.2.2 AND 25.5.2, RESPECTIVELY.
2. CASES 1 & 2 WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER AND THE CENTER-TO-CENTER SPACING OF BARS ARE DEFINED IN THE TABLE ON THE RIGHT.
3. LAP SPLICE LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS: CLASS A - 1.0 AND CLASS B - 1.3 (ACI 25.5.2.1). VALUES FOR BARS IN BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT MEETING MINIMUM REQUIREMENTS FOR STIRRUPS IN ACI 9.6.3 AND 9.6.4 OR MEETING THE REQUIREMENTS OF ACI 10.6.2 AND ARE BASED ON MINIMUM COVER SPECIFIED IN ACI 20.6.1.
4. CONDITIONS WHICH REQUIRE CASE 1 SPLICE LENGTHS SHOULD BE AVOIDED IF AT ALL POSSIBLE FOR THE LARGER BAR SIZES. THESE ORDINARILY LONG LENGTHS PRESENT POSSIBLE CONSTRUCTABILITY PROBLEMS DUE TO PLACING CONGESTION ETC.
5. TOP BARS ARE HORIZONTAL WITH MORE THAN 12 IN. OF FRESH CONCRETE CAST BELOW THE BARS.

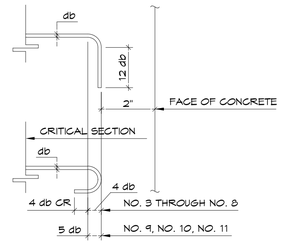
NORMAL WEIGHT CONCRETE STRENGTH	DEVELOPMENT LENGTH	LAP SPLICE LENGTH
3,000 PSI	22 db	30 db ≥ 12"
4,000 PSI	19 db	30 db ≥ 12"
5,000 PSI	18 db	30 db ≥ 12"
6,000 PSI	17 db	30 db > 12"

COMPRESSION DEVELOPMENT AND LAP SPLICE LENGTHS

NORMAL WEIGHT CONCRETE STRENGTH	BASIC HOOK DEVELOPMENT LENGTH
3,000 PSI	22 db
4,000 PSI	19 db
5,000 PSI	17 db

STANDARD HOOKED BAR TENSION DEVELOPMENT LENGTHS

1. WHEN COVERAGE REQUIREMENTS SHOWN IN DETAILS ARE MET, BASIC HOOK DEVELOPMENT LENGTH MAY BE MULTIPLIED BY 0.7.



REINFORCING DEVELOPMENT LENGTH AND LAP SPLICE SCHEDULE

JT ENGINEERING
Building Solutions
1321 Brunswick Ave, Lawrence, NJ 08648
161 Washington Street, 13th Floor, Suite 1375 Conshohocken, PA 19428
P: 609.303.0236
F: 609.303.0237
www.jt-pe.com

JASON C. TARANTINO
Jason Tarantino
Digitally signed by Jason Tarantino
Date: 2023.12.26 17:19:49 -05'00'
FLORIDA LICENSE: PE 94381

ISSUED FOR PERMIT

REVISIONS:

Mk	DATE:	REMARKS
1	12/08/23	ISSUED FOR PERMIT

PROJECT:
Proposed Building
Okeechobee (Shul)
FL-78 & Access Road
Okeechobee, Florida, 34978

ALL DRAWINGS AND WRITTEN MATERIALS APPEARING HEREIN CONSTITUTE ORIGINAL AND UNPUBLISHED WORK OF THE ENGINEER. THE REPRODUCTION OF THIS DRAWING FOR THE PURPOSE OF COPYING THIS WORK OR REVISING SAID DRAWING SHALL BE CONSIDERED A VIOLATION OF BOTH THE PROFESSIONAL CODE OF ETHICS AND A THEFT OF COMPANY ASSETS, BOTH OF WHICH SHALL BE PROSECUTED TO THE FULLEST EXTENT OF CURRENT STATUTES.

DRAWING TITLE:
SCHEDULES

DRAFTED BY: NAR
REVIEWED BY: JCT
PROJECT NUMBER: 2318_05
DRAWING SCALE: AS NOTED
DRAWING NUMBER: S005

ARCHITECTS