Written dimensions on these drawings shall have precedence over scaled dimensions. Contractor shall verify & be responsible for all dimensions & conditions on the job. Gutherman Structural, Inc. must be notified immediately in writing of any variation from the dimensions & conditions shown on these drawings. Field verify all conditions prior to laying out or fabricating associated work. Bring to the Structural Engineer's attention any deviation from designed conditions & field conditions prior to ordering materials & installing work. Shop details must be submitted to the our office & reviewed by Gutherman Structural, Inc. prior to ordering materials, fabrication & delivery to the job site, unless otherwise provided for under the specific contractual agreement. Gutherman Structural, Inc. expressly reserves its common law copyright & other property rights in these documents. These documents are not to be reproduced, changed, or copied in any form or manner whatsoever, nor are they to be assigned to a third party without first obtaining the written permission & written consent of Gutherman Structural, Inc. In the event of unauthorized use of these documents by a third party, the third party shall hold Gutherman Structural, Inc. harmless & agrees to remunerate Gutherman Structural, Inc. for such use in an amount equal to the original fee for the original documents, plus legal fees, court costs, collection fees & other costs.

ALWAYS REFER TO THE LATEST DRAWING SET. EACH CONTRACTOR SHALL COORDINATE THEIR WORK WITH ALL OTHER TRADES.

STRUCTURAL NOTES

CONTRACTOR NOTE:

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. GUTHERMAN STRUCTURAL, INC. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

CODES AND STANDARDS

1. WIND LOADS AS PER:

   A. FLORIDA BUILDING CODE 8TH EDITION (2023) WITH AN ULTIMATE DESIGN WIND SPEED OF 150 MPH, EXPOSURE C, NOMINAL DESIGN WIND SPEED OF 116 MPH, +/-0.18 INTERNAL PRESSURE COEFFICIENT, AND BUILDING RISK CATEGORY II. PARTIALLY ENCLOSED BUILDING.

2. SEISMIC SITE CLASS = E
   SEISMIC DESIGN CATEGORY = A
   GROUND SNOW LOAD, Pg = 0 PSF
   FLOOD ZONE = X
   RAIN INTENSITY = 4.0 INCHES PER HOUR (100 YEAR)

3. THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:

   A. FLORIDA BUILDING CODE 8TH EDITION (2023).
   B. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318/ 2019 EDITION).
   C. NATIONAL DESIGN SPECIFICATION, WOOD CONSTRUCTION NDS/2018 EDITION. DESIGN USING ASD (ALLOWABLE STRESS DESIGN) METHOD.
   D. BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (TMS 402, 602/2016 EDITION).

4. SECTIONS AND DETAILS:

   ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

FOUNDATION

1. BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2,000 PSF. SOILS SHALL BE COMPACTED TO 95% MODIFIED PROCTOR.

2. SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING WORK. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.

3. ALL MONOLITHIC EXTERIOR EDGE FOOTINGS SHALL BEAR A MINIMUM 1'-0" (PER SOILS REPORT) BELOW EXTERIOR GRADE TYP. MAKE ADJUSTMENTS AS NEEDED. INTERIOR FOOTINGS SHALL BEAR A MINIMUM 1'-0" THICK (PER SOILS REPORT) BELOW EXTERIOR GRADE TYP.

4. EXCAVATION & BACKFILL:

   A. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.

   B. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.

CONCRETE

1. ALL CONCRETE SHALL BE READY MIX 3,000 PSI AND MEET THE FOLLOWING REQUIREMENTS:

   A. SLUMPS SHALL BE 4-INCHES MINIMUM AND 6-INCHES MAXIMUM.
   B. CONCRETE SHALL HAVE 3 PERCENT AIR ENTRAINMENT.
   C. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.45.
   D. JOBSITE WATER SHALL NOT BE ADDED.
   E. CEMENT SHALL CONFORM WITH ASTM C150 TYPE 1. SLAG, ASTM C989 SHALL BE LIMITED TO 50% (BY WEIGHT OF CEMENTITIOUS MATERIAL AND FLY ASH, ASTM C618, CLASS F, SHALL BE LIMITED TO 25% (BY WEIGHT) OF CEMENTITIOUS MATERIAL.

2. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE (ACI 318/ 2019 EDITION), THE ACI DETAILING MANUAL (ACI 315R-2018), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301/2020).

3. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.

4. WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 1064, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.

5. LAP ALL BARS WITH CLASS B TENSION LAP SPLICE UNLESS OTHERWISE NOTED ON DRAWINGS. LAP ALL WWF A MINIMUM OF 12 INCHES (UNLESS OTHERWISE NOTED).

6. REINFORCING BARS:

   A. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A 615 GRADE 60.

   B. AT CORNERS OF CONCRETE WALLS, BEAMS AND CONTINUOUS WALL FOOTINGS, PROVIDE (1-#5 OR MATCHING) HORIZONTAL BARS X 5'-0" BENT BAR FOR EACH HORIZONTAL BAR SCHEDULED AT EACH FACE.

   C. ALL HOOKS SHOWN IN REINFORCEMENT SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.

MASONRY

1. MASONRY UNITS SHALL BE LOAD BEARING ASTM C90, NORMAL WEIGHT LAID IN A FULL BED OF MORTAR IN RUNNING BOND.

2. THE COMPRESSIVE STRENGTH OF MASONRY (F'M) SHALL BE 2,000 PSI AS CALCULATED IN ACCORDANCE WITH ASTM C-140.

3. ALL MORTAR SHALL BE IN ACCORDANCE WITH ASTM SPECIFICATION C270 HAVING A MINIMUM COMPRESSIVE STRENGTH OF (S) 1,800 PSI OR (M) 2,500 PSI

4. GROUT SHALL BE A HIGH SLUMP MIX IN ACCORDANCE WITH ASTM SPECIFICATION C476, HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

5. ALL MASONRY CONSTRUCTION AND INSPECTIONS SHALL BE IN ACCORDANCE WITH THE LATEST ACI BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES TMS602/ACI530.1/ASCE 6, ASTM C476-23M ASTM C1019-20.

6. PROVIDE 8" X 8" MASONRY BEAM WITH 1 #5 CONT. AT EVERY WINDOW SILL. EXTEND BEAM 8" BEYOND EDGE OF OPENING.

7. ALL VERTICAL REINFORCING SHALL BE HOOKED AT TOP AND BOTTOM AT ALL VERTICAL REINFORCED CELL RUNS. THIS INCLUDES IN BEAMS AND LINTELS OVER OPENINGS.

8. THE GENERAL CONTRACTOR SHALL PROVIDE AND INSTALL BRACING FOR ALL MASONRY WALLS AS REQUIRED TO ENSURE STABILITY DURING CONSTRUCTION.

9. PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS. PROVIDE DOVE TAIL SLOT ANCHORS AT CONCRETE COLUMNS. LAP ALL WIRE 6" MIN. TYP.
   ALL JOINT REINFORCEMENT, WALL TIES, ANCHORS AND INSERTS SHALL BE HOT DIP GALVANIZED.

10. MINIMUM LAP SPLICES FOR REINFORCED MASONRY (40 BAR DIA. MIN.):

BAR SIZE			
#4	#5	#6	#7
20"	25"	30"	35"

   A. LAP SPLICES SHALL OCCUR DIRECTLY ABOVE FOOTINGS AND SLABS. NO SPLICES ARE ALLOWED AT MID-HEIGHT OF WALL.

   B. LAP SPLICES THAT OCCUR AT CANTILEVERED WALLS SUCH AS: PARAPETS, RETAINING WALLS, ETC. SHALL HAVE LAP SPLICE LENGTHS INCREASED BY 50%.

11. MASONRY LINTELS:

   A. A PRECAST CONCRETE LINTEL BY PRE-CAST MANUF. SHALL BE PROVIDED OVER ALL MASONRY WALL OPENINGS. THE LINTEL SHALL BE FULLY GROUTED.

   B. LINTELS TO HAVE 4" MINIMUM BEARING AT EACH END.

   C. SHORE PRECAST LINTEL PER MANUFACTURE'S INSTRUCTIONS.

EXPANSION ANCHORS, ADHESIVE ANCHORS AND POWER ACTUATED FASTENERS

A. ALL POST INSTALLED ANCHORS SHALL BE APPROVED BY THE EOR PRIOR TO INSTALLATION.

ALL PROPRIETARY ANCHORING SYSTEMS (EXPANSION, ADHESIVE ANCHORING SYSTEMS ETC) TO BE INSTALLED INTO EXISTING CONCRETE AND MASONRY ELEMENTS ARE TO BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS FOR DRILLING AND CLEANING OF HOLES, FOR SPACING AND EDGE DISTANCE REQUIREMENTS, AND FOR THE UTILIZATION OF SUPPLEMENTAL COMPONENTS FOR THE ANCHORING SYSTEMS SUCH AS SCREEN TUBES, DOWELING ADHESIVES, ETC.

B. FOR CONNECTIONS TO EXISTING CONCRETE AND MASONRY, CONTRACTOR MUST LOCATE THE POSITION OF EXISTING REINFORCING BARS WITH AN R-METER OR PILOT HOLES PRIOR TO INSTALLATION OF ANCHORS. NOTIFY ENGINEER OF FIELD CONFLICTS PRIOR TO INSTALLATION.

C. CONNECTIONS TO EXISTING CONCRETE SHALL BE MADE WITH ANCHORS CONFORMING TO ACI 318, CHAPTER 17 AND THE TMS 402/602-16, CHAPTER 6.3.

1. MECHANICAL ANCHORS SHALL BE EITHER:
-HILTI "KWIK BOLT TZ2"
-HILTI "KWIK BOLT 1"
-HILTI "KWIK HUS-EZ SCREW ANCHOR (USE ONLY IN PERMANENTLY DRY INTERIOR NON-CORROSIVE ENVIRONMENTS)
-SIMPSON "STRONG BOLT 2"
-SIMPSON "TITEN HD" SCREW ANCHOR (USE ONLY IN PERMANENTLY DRY, INTERIOR NON-CORROSIVE ENVIRONMENTS)

2. ADHESIVE ANCHOR RODS OR REINFORCING BARS SHALL BE INSTALLED IN ROTARY HAMMERED DRILLED HOLES USING THE FOLLOWING ADHESIVE ANCHORING SYSTEM:
-HILTI "HIT HY 200" SAFE SET SYSTEM WITH HILTI HIT-Z RODS OR WITH ISO 898 CLASS 5.8 ANCHOR RODS (MINIMUM YIELD STRENGTH = 58 KSI AND MINIMUM ULTIMATE STRENGTH = 72.5 KSI) OR ASTM A193 GRADE B7 HIGH STRENGTH ANCHOR RODS.
-HILTI "HIT RE 500" ADHESIVE ANCHORING SYSTEM WITH ISO 898 CLASS 5.8 ANCHORS RODS (MINIMUM YIELD STRENGTH = 58 KSI AND MINIMUM ULTIMATE STRENGTH = 72.5 KSI) OR ASTM A193 GRADE B7 HIGH STRENGTH ANCHOR RODS.

-SIMPSON "SET-3G" ASHESIVE ANCHORING SYSTEM WITH ASTM F1554 GRADE 36 ANCHOR RODS OR ASTM A193 GRADE B7 HIGH STRENGTH ANCHOR RODS.
-SIMPSON "AT-3G" ADHESIVE ANCHORING SYSTEM WITH ASTM F1554 GRADE 36 ANCHOR RODS OR ASTM 193 GRADE B7 HIGH STRENGTH ANCHOR RODS.

USING ANY OF THE ABOVE ADHESIVE ANCHORING SYSTEMS, REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF THE CONCRETE GENERAL NOTES. SIZE EMBEDMENT, SPACING AND EDGE DISTANCE OF ANCHORS AND REINFORCING BARS SHALL BE AS INDICATED ON THE DRAWINGS.

D. CONNECTIONS TO GROUT FILLED CONCRETE MASONRY SHALL BE MADE WITH HILTI STANDARD "HAS-E" ISO 898 CLASS 5.8 ANCHOR RODS (MINIMUM YIELD STRENGTH = 58KSI AND MINIMUM ULTIMATE STRENGTH = 72.5 KSI) USING HILTI "HIT-HY 270"ADHESIVE ANCHORING SYSTEMS OR EQUAL AS APPROVED BY THE ENGINEER.

E. EMBED ANCHOR RODS INTO DRILLED HOLES A MINIMUM OF 9 ANCHOR DIAMETERS, WITH A MINIMUM EDGE DISTANCE OF 4 INCHES UNLESS OTHERWISE NOTED, MEASURED FROM THE EDGE OF THE MASONRY TO THE CENTERLINE OF THE ANCHOR. INCREASED EMBEDMENT DEPTHS OR EDGE DISTANCES MAY BE REQUIRED AT CERTAIN LOCATIONS. SEE PLAN DETAILS.

F. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI (ACI318-11 D9.2.2)/ (ACI318-19 - 26.13.1.5 & 26.13.1.6) PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.

G. AHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS (ACI318-19 17.2.2)

H. ALL POWDER ACTUATED FASTENERS SALL BE 0.177" SHANK DIAMETER PINS BY HILTI OR APPROVED EQUAL. PIN LENGTHS SHALL BE SUFFICIENT TO ALLOW FULL PENETRATION THROUGH THE STRUCTURAL STEEL. PIN SPACING SHALL BE LESS THAN 1 1/2" AND EDGE DISTANCE SHALL BE NO LESS THAN 1/2"

WOOD

1. ALL STRUCTURAL WOOD MEMBERS ARE DESIGNED AS "DRY-USE". MOISTURE CONTENT MUST BE 19% OR LESS. STORE WOOD FRAMING ABOVE GROUND AND UNDER TARPS WITH PROPER AIR CIRCULATION.

2. ALL LUMBER SHALL BE SOUTHERN PINE SPECIES #2 GRADE OR APPROVED EQUAL. ALLOWABLE DESIGN STRESSES SHALL FOLLOW NATIONAL DESIGN SPECIFICATION (NDS) (LATEST EDITION).

3. HEADERS AT NON-BEARING CONDITIONS SHALL BE AS FOLLOWS:

OPENING SIZE	HEADER AT 2x4 WALL	HEADER AT 2x6 WALL
UP TO 6' -0"	(2) 2" X 4"	(3) 2" X 4"
4'- 0" TO 6'- 0"	(2) 2" X 6"	(3) 2" X 4"
6'- 0" TO 9'- 0"	(2) 2" X 8"	(3) 2" X 6

   ALL NON-LOAD BEARING INTERIOR UNIT WALLS SHALL NOT BE BUILT TIGHT TO THE BOTTOM OF THE FLOOR TRUSS CEILING MEMBERS. UTILIZE THE SIMPSON STRONG-DRIVE SDPW DEFLECTOR SCREW TO ALLOW FOR A MINIMUM 3/4" GAP BETWEEN TRUSS BOTTOM CHORD AN THE NON-LOAD BEARING PARTITION WALL TOP PLATE. SCREW SHALL BE INSTALLED AT EACH WALL RUNNING PERPENDICULAR TO THE FLOOR/ROOF TRUSS ABOVE.

4. PROVIDE SP ACQ PRESSURE TREATED LUMBER IN ACCORDANCE WITH AWPA STANDARDS TO A MINIMUM 0.40 PCF RETENTION WHERE LUMBER IS IN CONTACT WITH CONCRETE/MASONRY OR OUTSIDE OF BUILDING. ALL METAL CONNECTORS IN CONTACT WITH PRESSURE TREADED LUMBER SHALL BE GALVANIZED WITH A RATING OF G-185 AND CONFORM TO ASTM A653. ALL NAILS AND SCREWS USED WITH PRESSURE TREATED LUMBER ARE TO BE HOT-DIPPED GALVANIZED AND TO CONFORM TO ASTM A153 CLASS D. ELECTROGALVANIZED FASTENERS SHALL HAVE A CLASS RATING PER ASTM B695 NO LESS THAN 55. ALUMINUM NOT TO BE USED IN DIRECT CONTACT WITH ACQ TREATED LUMBER.

5. PLYWOOD SHEATHING:

   A. ROOF: Use (19/32"-40/20 RATED), EXP. 1, PLYWOOD/OSB SHEATHING. ATTACH TO TRUSSES WITH 8d OR 10d RING SHANK NAILS AT 6" O.C. AT PANEL EDGES, AND 6" O.C. ALL OTHER SUPPORTS. H-CLIPS SHALL BE PROVIDED ON ALL UNBLOCKED PANEL EDGES, CENTRED BETWEEN SUPPORTS, (24" O.C.)

   B. SEE FRAMING PLANS FOR NAILING AND/OR BLOCKING REQUIREMENTS. USE 8'- 0" LONG X 4'-0" WIDE SHEETS WITH LENGTH ACROSS FRAMING. STAGGER PANEL END JOINTS 4'-0" TYP., ALLOW 1/8" SPACE ALONG PANEL EDGES AND END JOINTS.

   C. PLYWOOD FASTENER SIZES:

   ROOF SHEATHING — 8d COMMON (2 1/2"x 0.131 x 0.281) OR 8d GALV BOX NAIL (2 1/2"x 0.113 x 0.297)

6. WOOD CONNECTIONS - ALL NAILS USED FOR STRUCTURAL FRAMING MEMBERS SHALL BE COMMON WIRE, U.N.O. ALL FLAT CS TYPE METAL STRAPS MUST BE INSTALLED WITH EQUAL LENGTHS ABOUT THE JOINT LINE. USE SIMPSON STRONG-TIE CONNECTOR PRODUCTS OR APPROVED EQUAL. TOE NAILING WILL NOT BE PERMITTED.

FOOTING SCHEDULE

MARK	SIZE	REINFORCING
MF-1	1'-6" X 24" X CONT.	(3) #5 CONT. BOTTOM #5 AT 12" O.C. TRANS. BOTTOM
TS30	3'-0" X 3-0"x24"	(4) #5 E.W. BOTTOM

DESIGN LOAD SCHEDULE

(ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)

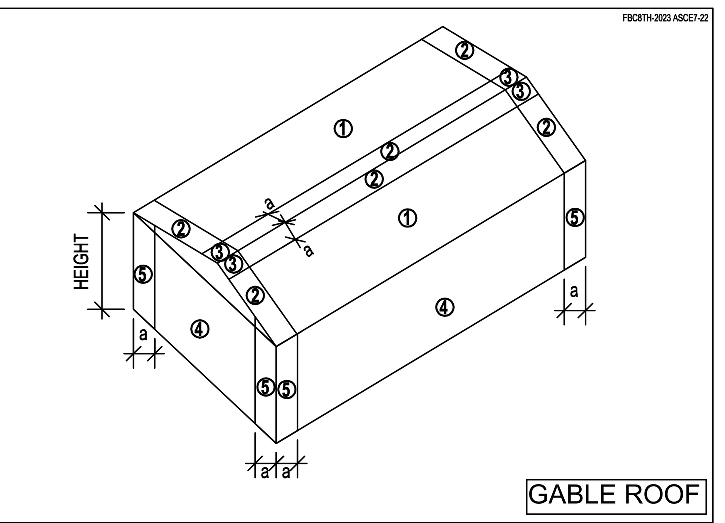
COMPONENT \ AREA	ROOF	SLAB ON GRADE			
SYSTEM	15				
RAIN LOAD	0				
TOTAL DEAD LOAD	15				
TOTAL LIVE LOAD	20	100			
TOTAL LOAD	35	100			

WIND PRESSURES (PSF) COMPONENTS AND CLADDING (VASD)

FBC8TH-2023 ASCE7-22

1. + INDICATES WIND PRESSURE
   - INDICATES WIND SUCTION
2. WALL DISTANCE
   a = *3.0FT* *(COMPONENTS & CLADDING)*
3. FOR DOORS, WINDOWS, & OTHER COMPONENTS + CLADDING, IF THE AREA IS BETWEEN THOSE SHOWN ON THE DIAGRAM USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE WIND AREA.
4. N/A INDICATES NOT APPLICABLE

ROOF AREA ①	ROOF AREA ②	ROOF AREA ③	EFFECTIVE WIND AREA (SQ. FT.)	WALL AREA ④	WALL AREA ⑤
+26.5/-53.7	+27.0/-71.2	+27.0/-90.2	10	+36.3/-38.5	+36.3/-45.3
FOR JOIST UPLIFT LOADS, USE THE VALUES SHOWN USING -5 PSF FOR DEAD LOAD			20	+35.1/-37.3	+35.1/-42.9
			50	+33.5/-35.7	+33.5/-39.7
			100	+32.3/-34.5	+32.3/-37.3



GABLE ROOF

BEAM SCHEDULE

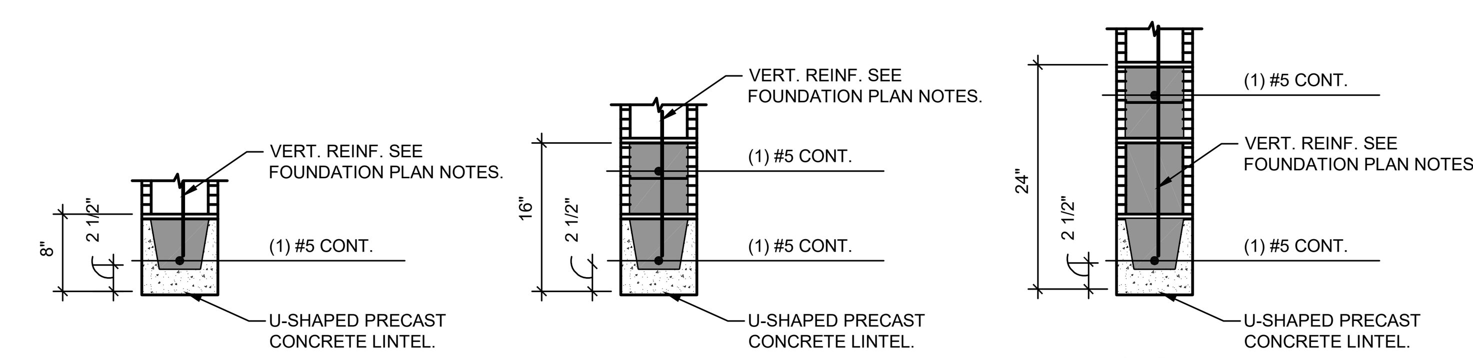
MARK	SIZE BxH (inches)	REINFORCING BOTT	REINFORCING TOP	REINFORCING INT	STIRRUPS TIES SPACING
BB-1	8x8	(1) #5	-	-	MASONRY BOND BEAM
BB-2	8x16	(1) #5	(1) #5	-	MASONRY BOND BEAM

MASONRY WALL/COLUMN SCHEDULE

MARK	THICKNESS	REINFORCING
MW-1	8" CMU	#5 @ 48" O.C.
MW-2	8" CMU	#5 @ 8" O.C.
MC	8" CMU COLUMN	(1) #5 VERT. AT EA. CELL

MASONRY WALL NOTES:

1. WALL SEGMENTS SHALL BE REINFORCED WITH 9 GA. GALVANIZED LATERAL REINFORCING @ 16" O.C. HORIZ. EXTEND REINFORCING 6" INTO POURED ELEMENTS AND AROUND ENCASED STEEL.
2. ADJACENT TO ANY EXTERIOR/INTERIOR 8" WALL OPENING, PLACE 2 #6 VERTICAL (ONE PER CELL) IN CELL GROUTED SOLID, FULL HEIGHT, U.N.O.
3. ALL MASONRY REINFORCED CELLS SHALL BE FILLED WITH 3000 PSI GROUT MIX.



L-1 (8F8-1B)

L-2 (8F16-1B/1T)

L-3 (8F24-1B/1T)

C.M.U. PRECAST LINTEL SCHEDULE

N.T.S.

NOTES:

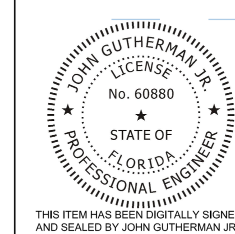
1. 8" PRECAST LINTELS BY CASTCRETE CORPORATION OR APPROVED EQUAL.
2. SHORE PRECAST LINTEL PER MANUFACTURER RECOMMENDATIONS.
3. SEE THE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS.
4. PROVIDE 4" MINIMUM BEARING EACH END.

To the best of the engineer's knowledge, the plans & specifications comply with the applicable minimum building codes & the applicable fire-safety st&ards as determined by the local authority.
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LIC. # 28863

SUBMISSIONS

#	DATE	DESCRIPTION



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Digitally signed by John Gutherman Jr.
Date: 2026.07.03 10:56:14-04'00'

SHEET TITLE

GENERAL NOTES

SHEET NUMBER

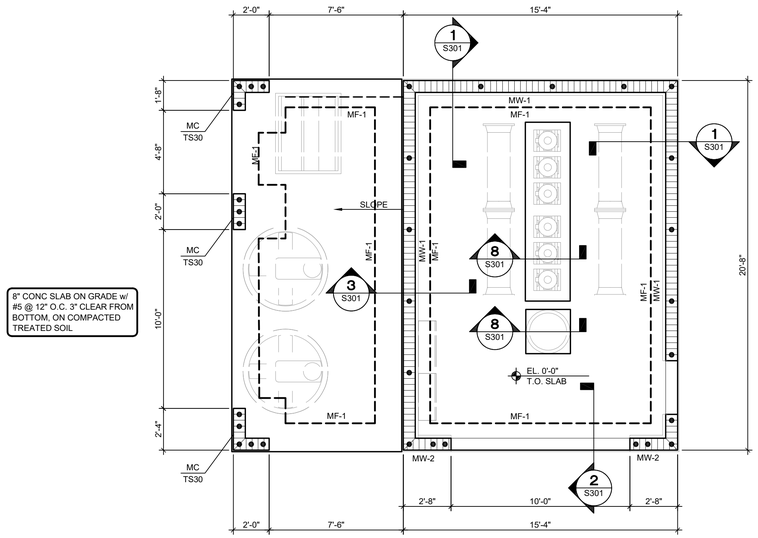
S001

DRAWN BY :

CHECKED BY :

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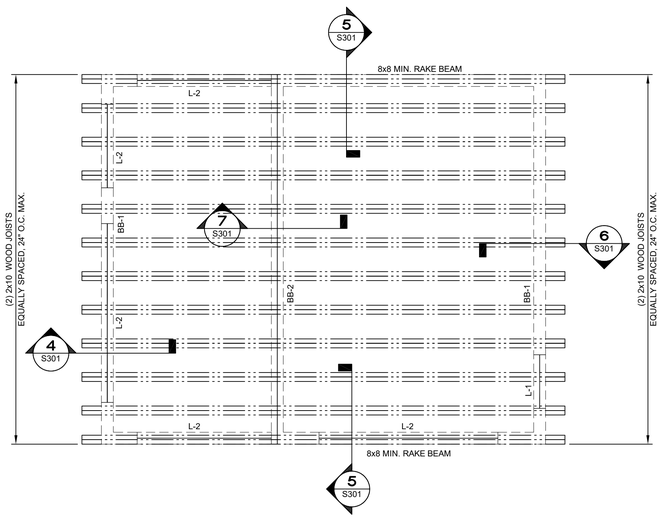
1 Foundation Plan

1/4" = 1'-0"

NORTH

FOUNDATION NOTES:

1. ELEV. ±0'-0" IS REFERENCE ONLY. SEE CIVIL FOR TRUE NAVD ELEVATION.
2. VERIFY SLOPES AND STEPS WITH ARCH'L PRIOR TO CONSTRUCTION. SEE TYPICAL STEP DETAIL ON S201.
3. MF## INDICATES MONOLITHIC CONCRETE WALL FOOTING. SEE S001 FOR SCHEDULE.
4. MW-# INDICATES CONCRETE MASONRY WALL, SEE SCHEDULE ON S001.
5. ▭ DENOTES 8" CMU WALL, SEE SCHEDULE ON S001. UNLESS NOTED OTHERWISE ON PLAN.
6. SLABS SHALL BE 8" CONCRETE ON GRADE w/ #5 @ 12" O.C. 3" CLEAR FROM BOTTOM, ON COMPACTED TREATED SOIL



2 Roof Framing Plan

1/4" = 1'-0"

NORTH

ROOF FRAMING NOTES:

1. L-# INDICATES PRECAST LINTEL, SEE SCHEDULE ON S001.
2. PROVIDE A 8X8 MASONRY BOND BEAM (BB-1) OR (BB-2) AT THE TOP OF ALL WALLS.
3. TOP OF MASONRY WALL ELEVATIONS SHALL BE 8'-8"
4. VERIFY ALL BOTTOM OF BEAM/LINTEL ELEVATIONS W/ ARCH DRAWINGS PRIOR TO FABRICATION AND CONSTRUCTION, TYP.
5. 2-PLY RAFTER ATTACHMENTS: 10d NAILS AT 12" ALONG TOP AND BOTTOM. 1.5" CLEAR FROM EDGES.

To the best of the engineer's knowledge, the plans & specifications comply with the applicable minimum building codes & the applicable fire-safety standards as determined by the local authority.

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Gutherman STRUCTURAL

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LIC. # 28863

SUBMISSIONS

#	DATE	DESCRIPTION
	07/03/2026	PERMIT SET

JOHN GUTHERMAN JR. LICENSE No. 60880 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY JOHN GUTHERMAN JR, PE ON THE DATE ADJACENT TO THE SEAL.

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Date: 2026.07.03 10:56:17-04'00'

LAKEFRONT ESTATES DEVELOPMENT

POTABLE WATER STORAGE AND

GLADES COUNTY, FL

SHEET TITLE

STRUCTURAL PLANS

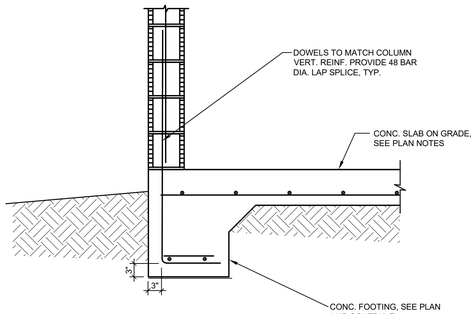
SHEET NUMBER

S101

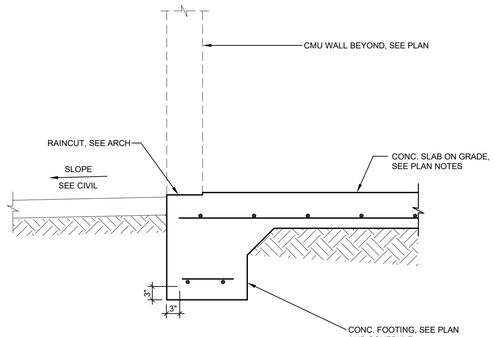
DRAWN BY : RED

CHECKED BY : JG

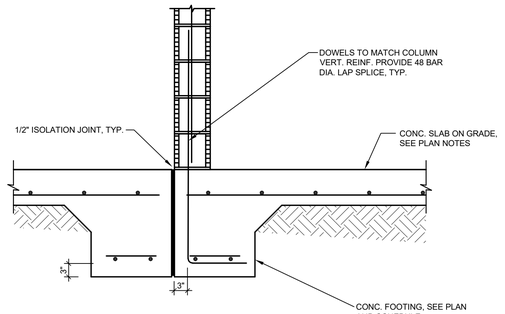
Written dimensions on these drawings shall have precedence over scaled dimensions. Contractor shall verify & be responsible for all dimensions & conditions on the job. Gutherman Structural, Inc. must be notified immediately in writing of any variation from the dimensions & conditions shown on these drawings. Field verify all conditions prior to laying out or fabricating associated work. Bring to the Structural Engineer's attention any deviation from designed conditions & field conditions prior to ordering materials & installing work. Shop details must be submitted to the our office & reviewed by Gutherman Structural, Inc. prior to ordering materials, fabrication & delivery to the job site, unless otherwise provided for under the specific contractual agreement. Gutherman Structural, Inc. expressly reserves its common law copyright & other property rights in these documents. These documents are not to be reproduced, changed, or copied in any form or manner whatsoever, nor are they to be assigned to a third party without first obtaining the written permission & written consent of Gutherman Structural, Inc. In the event of unauthorized use of these documents by a third party, the third party shall hold Gutherman Structural, Inc. harmless & agrees to remunerate Gutherman Structural, Inc. for such use in an amount equal to the original fee for the original documents, plus legal fees, court costs, collection fees & other costs.



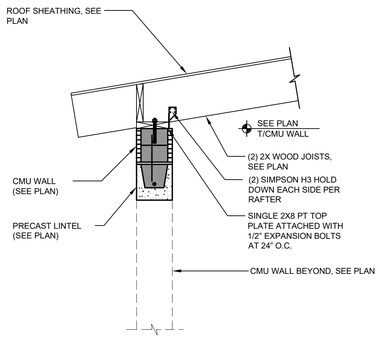
SECTION 1 — S301
3/4" = 1'-0"



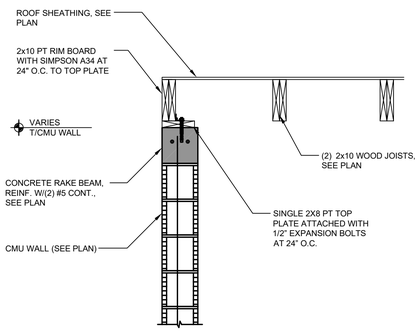
SECTION 2 — S301
3/4" = 1'-0"



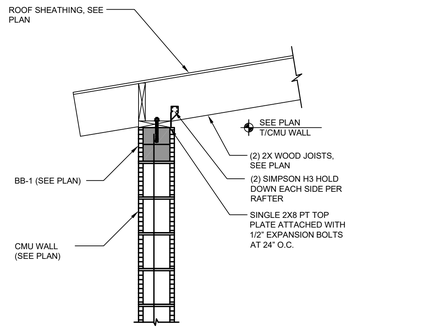
SECTION 3 — S301
3/4" = 1'-0"



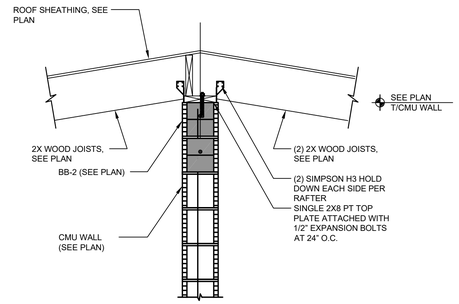
SECTION 4 — S301
3/4" = 1'-0"



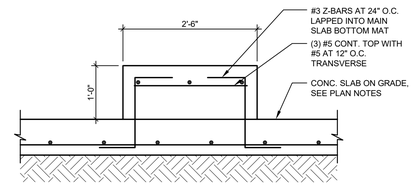
SECTION 5 — S301
3/4" = 1'-0"



SECTION 6 — S301
3/4" = 1'-0"



SECTION 7 — S301
3/4" = 1'-0"



SECTION 7 — S301
3/4" = 1'-0"

ALWAYS REFER TO THE LATEST DRAWING SET. EACH CONTRACTOR SHALL COORDINATE THEIR WORK WITH ALL OTHER TRADES.

To the best of the engineer's knowledge, the plans & specifications comply with the applicable minimum building codes & the applicable fire-safety stdards as determined by the local authority.
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SUBMISSIONS

#	DATE	DESCRIPTION
	07/03/2026	PERMIT SET

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Date: 2026.07.03 10:56:20-04'00'

LAKEFRONT ESTATES DEVELOPMENT
POTABLE WATER STORAGE AND
GLADES COUNTY, FL

SHEET TITLE
STRUCTURAL SECTIONS

SHEET NUMBER
S301

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CHECKED BY : JG